ATTACHMENT 4

## FLOW MINIMIZATION ALTERNATIVES

 THROUGH-SCREEN VELOCITIES \&LONG-TERM WEST BASIN WATER LEVEL ANALYSIS FOR ASSESSING TRESHOLD IMPINGEMENT EFFECTS OF REDUCED INTAKE FLOWS AT AGUA HEDIONDA LAGOON

FLOW MINIMIZATION ALTERNATIVES - THROUGH-SGREEN VELOCITY ASSESSMENT
existing intake pumps, screens and screen velocities


CHANNEL AND SCREEN VELOCITIES WITH ALL PUMPS IN OPERATION - TOTAL INTAKE FLOW OF 794.92 MGD

| Channels for Units 1,2 \& 3 |  | Total | Unit 1, 2 \& 3 Flow $=$ |  | 322.2 | cfs | 322.2 cfs (check) |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Channel Bottom Elevation = | -20 |  | $\begin{array}{\|l\|} \hline \text { Low Tide } \\ \text { In-Channel } \end{array}$ | Velocity $=$ | 1.20 | fps | Low Tide Through-Screen Velocity $=$ | 2.10 |
| Channel Width $=$ | 12.5 |  | $\mathrm{R}=$ |  |  |  |  |  |
| Channel Depth $=$ | 25 |  |  |  | 5.7737839 |  |  |  |
| Water Depth in Channels (Low Tide) | 10.73 |  |  |  |  |  |  |  |
| Water Depth in Channels (High Tide) | 19.93 |  |  |  |  |  |  |  |
|  |  |  | $\begin{aligned} & \text { High Tide } \\ & \text { in-Channe! } \\ & \hline \end{aligned}$ | Velocity $=$ | 0.65 | fps | High Tide <br> Through-Screen Velocity $=$ | 1.13 |
| Channel for Unit 4 <br> Number of Screen Channels $=$ | 2 | Total | Unit 4 Flow $=$ |  | 446.0 | cfs | 446 cfs (check) |  |
| Channel Bottom Elevation = | -20 |  | $\begin{aligned} & \text { Low Tide } \\ & \text { In-Channel } \end{aligned}$ | Velocity $=$ | 1.88 | fps | Low Tide <br> Through-Screen Velocity $=$ | 3.29 |
| Channel Width $=$ | 11.25 |  | $\mathrm{R}=$ |  | 5.439 |  |  |  |
| Channel Depth $=$ | 25.75 |  |  |  |  |  |  |  |


| Water Depth in Channels (Low Tide) | 10.53 |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Water Depth in Channels (High Tide) | 19.43 |  |  |  |  |  |  |  |
|  |  |  | $\begin{aligned} & \text { High Tide } \\ & \text { In-Channel } \end{aligned}$ | Velocity $=$ | 1.02 | fps | $\begin{aligned} & \text { THigh Tide } \\ & \text { Through-Screen Velocily = } \end{aligned}$ | 1.79 |
| Channel for Unit 5 Number of Screen Channels = | 3 | Total | Unit 5 Flow $=$ |  | 463.84 | cfs | 463.84 cfs (check) |  |
| Channel Bottom Elevation $=$ | -20 |  | $\begin{aligned} & \text { Low Tide } \\ & \text { In-Channel } \end{aligned}$ | Velocity $=$ | 1.33 | fps | Low Tide <br> Through-Screen Velocity = | 1.94 |
| Channel Depth $=$ | 27.75 |  |  | $\mathrm{R}=$ | 6.407599706 |  |  |  |
| Water Depth in Channels (Low Tide) | 10.33 |  |  |  |  |  |  |  |
| Water Depth in Channels (High Tide) | 19.23 |  |  |  |  |  |  |  |
|  |  |  |  | High Tide In-Channel | Velocity $=$ | 0.71 | fps | High Tide Through-Screen Velocity $=$ | 1.04 |



OPERATIONAL CONDITION 2 - TOTAL INTAKE FLOW = 322.58 MGD


OPERATIONAL CONDITION 3 - TOTAL INTAKE FLOW = $\mathbf{3 2 8 . 3 3}$ MGD



OPERATIONAL CONDITION 5 - TOTAL INTAKE FLOW $=184.32$ MGD


| Channel for Unit 5 <br> Number of Screen Channels $=$ | 3 | Total | Unit 5 Flow |  | 231.74 | cfs | 231.92 cfs (check) |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Channel Bottom Elevation = | -20 |  | $\begin{aligned} & \text { Low Tide } \\ & \text { In-Channel } \end{aligned}$ | Velocity $=$ | 1.07 | fps | $\begin{aligned} & \text { Low Tide } \\ & \text { Through-Screen Velocity = } \end{aligned}$ | 1.57 |
| Channel Width $=$ | 11.25 |  |  |  |  |  |  |  |
| Channel Depth $=$ | 27.75 |  |  | $\mathrm{R}=$ | 4.64017 |  |  |  |
| Water Depth in Channels (Low Tide) | 6.40 |  |  |  |  |  |  |  |
| Water Depth in Channels (High Tide) | 15.3 |  |  |  |  |  |  |  |
|  |  |  | High Tide in-Channel | Velocity $=$ | 0.45 | fps | \|High Tide Through-Screen Velocity $=$ | 0.66 |

OPERATIONAL CONDITION 4 - TOTAL INTAKE FLOW $=\mathbf{2 1 8 . 8 8}$ MGD

| Unit 1,2 or 3 (Two Pumps) $=$ | 69.12 MGD | 107.04 cfs |
| :--- | ---: | :--- |
| Unit 5 (One Pump) $=$ | 149.76 MGD | 231.92 cfs |
| Total Pump Flow $=$ | 218.88 MGD | 338.96 cfs |


| Channels for Units $1,2 \& 3$ <br> Number of Screen Channels = | 2 |
| :--- | ---: |
| Channel Bottom Elevation = | -20 |
| Channel Width = | 12.5 |
| Channel Depth = | 25 |
| Water Depth in Channels (Low Tide) | 4.937 ft |
| Water Depith in Channels (High Tde) | 13.837 fl |



FLOW MINIMIZATION ALTERNATIVES - THROUGH-SCREEN VELOCITY ASSESSMENT
EXISTING INTAKE PUMPS, SCREENS AND SCREEN VELOCITIES

| Power Plant Unit Number |  | mp Size |  | Maximum V Upstream | ocity (fps) Screens | Maximum Through | ocity (fps) Screens | Size | Screens Number |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | gpm | MGD | cfs | $\begin{aligned} & \text { High Tide } \\ & 4.83 \end{aligned}$ | $\begin{aligned} & \text { Low Tide } \\ & -5.07 \end{aligned}$ | High Tide 4.83 | Low Tide <br> $-5.07$ | (in) |  |  |  |
| Unit 1 |  |  |  |  |  |  |  |  |  |  |  |
| Pump 15 | 24,000 | 34.56 | 53.52 |  |  |  |  |  |  |  |  |
| Pump 1 N | 24,000 | 34.56 | 53.52 |  |  |  |  |  |  |  |  |
| Total Pump Capacity Unit $1=$ | 48,000 | 69.12 | 107.04 | 0.7 |  | 1.2 | 2.1 | 3/8-Inches |  | 2 | 1.75 |
| Unit 2 |  |  |  |  |  |  |  |  | Shared w/ Units 1\&2 |  |  |
| Pump 2 S | 24,000 | 34.56 | 53.52 |  |  |  |  |  |  |  |  |
| Pump 2 N | 24,000 | 34.56 | 53.52 |  |  |  |  |  |  |  |  |
| Total Pump Capacity Unit 2 = | 48,000 | 69.12 | 107.04 | 0.7 |  |  | 2.1 | 3/8-inches |  | 2 | 1.75 |
| Unit 3 |  |  |  |  |  |  |  |  | Shared w/ Units 1 \& 3 |  |  |
| Pump 3 S | 24,000 | 34.56 | 53.52 |  |  |  |  |  |  |  |  |
| Pump 3 N | 24,000 | 34.56 | 53.52 |  |  |  |  |  |  |  |  |
| Total Pump Capacity Unit 3 = | 48,000 | 69.12 | 107.04 | 0.7 |  | 1.2 | 2.1 | 3/8-Inches |  | 2 | 1.75 |
| Unit 4 |  |  |  |  |  |  |  |  | Shared wl |  |  |
| Pump 4 E | 100,000 | 144.01 | 223 |  |  |  |  |  |  |  |  |
| Pump 4 W | 100,000 | 144.01 | 223 |  |  |  |  |  |  |  |  |
| Total Pump Capacity Unit $4=$ | 200,000 | 288.02 | 446 | 1.0 |  | 1.8 | 2.8 | 3/8-inches |  | 2 | 1.75 |
| Unit 5 |  |  |  |  |  |  |  |  |  |  |  |
| Pump 5E | 104,000 | 149.76 | 231.92 |  |  |  |  |  |  |  |  |
| Pump 5 W | 104,000 | 149.77 | 231.92 |  |  |  |  |  |  |  |  |
| Total Pump Capacity Unit 5 = | 208,000 | 299.54 | 463.84 | 0.7 | 1.1 | 1.0 | 1.6 | 5/8-Inches |  | 3 | 1.46 |

CHANNEL AND SCREEN VELOCITIES WITH ALL PUMPS IN OPERATION - TOTAL INTAKE FLOW OF 794.92 MGD

| Channels for Units 1,2 \& 3 Number of Screen Channels $=$ | 2 | Total | Unit 1, 2 \& 3 Flow $=$ |  | 322.2 | cfs | 322.2 cfs (check) |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Channel Bottom Elevation = | -20 |  | $\begin{aligned} & \text { Low Tide } \\ & \text { In-Channe! } \end{aligned}$ | Velocity $=$ | 1.20 | fps | Low Tide Through-Screen Velocity $=$ | 2.10 |
| Channel Width $=$ | 12.5 |  | $\mathrm{R}=$ |  | 5.7737839 |  |  |  |
| Water Depth in Channels (Low Tide) | 10.73 |  |  |  |  |  |  |  |
| Water Depth in Channels (Hight Tide) | 19.93 |  |  |  |  |  |  |  |
|  |  |  | $\begin{aligned} & \text { High Tide } \\ & \text { In-Channel } \end{aligned}$ | Velocity $=$ | 0.65 | fps | \|High Tide Through-Screen Velocity $=$ | 1.13 |
| Channel for Unit 4 Number of Screen Channels $=$ | 2 | Total | Unit 4 Flow $=$ |  | 446.0 | cfs | 446 cfs (check) |  |
| Channel Bottom Elevation $=$ | -20 |  | $\begin{aligned} & \text { Low Tide } \\ & \text { In-Channel } \end{aligned}$ | Velocily $=$ | 1.88 | fps | Low Tide <br> Through-Screen Velocity = | 3.29 |
| Channel Width = | 11.25 |  | $\mathrm{R}=\quad 5.439049587$ |  |  |  |  |  |
| Channel Depth $=$ | 25.75 |  |  |  |  |  |  |  |

Water Depth in Channels (Low Tide) $\quad 10.53$
Water Depth in Channels (Hight Tide) 19.43

| Channel for Unit 5 |  |
| :---: | :---: |
| Number of Screen Channels $=$ | 3 |
| Channel Bottom Elevation $=$ | -20 |
| Channel Width $=$ | 11.25 |
| Channel Depth $=$ | 27.75 |
| Water Depth in Channels (Low Tide) | 10.33 |


| $\begin{aligned} & \text { High Tide } \\ & \text { In-Channel } \end{aligned}$ | Velocity $=$ | 1.02 | fps | \|High Tide Through-Screen Velocity $=$ | 1.79 |
| :---: | :---: | :---: | :---: | :---: | :---: |
| Unit 5 Flow $=$ |  | 463.84 | cfs | 463.84 cfs (check) |  |
| $\begin{aligned} & \text { Low Tide } \\ & \text { Ln-Channel } \end{aligned}$ | Velocity $=$ | 1.33 | fps | $\begin{aligned} & \text { Low Tide } \\ & \text { Through-Screen Velocity = } \end{aligned}$ | 1.94 |
|  | $\mathrm{R}=$ | 6.40759 |  |  |  |
| High Tide |  |  |  | \|High Tide |  |

OPERATIONAL CONDITION 1 - TOTAL INTAKE FLOW $=\mathbf{3 1 6 . 9 6}$ MGD

| Unit 1 (Both Pumps) $=$ | 69.12 MGD | 107.04 cfs |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Unit 2 (One Pump) = | 34.56 MGD | 53.52 cfs |  |  |  |  |  |  |
| Unit 3 (Both Pumps) $=$ | 69.12 MGD | 107.04 cfs |  |  |  |  |  |  |
| Unit 4 (One Pump) = | 144.01 MGD | 223 cfs |  |  |  |  |  |  |
| Total Pump Flow = | 316.82 MGD |  | 6 cfs |  |  |  |  |  |
| Channels for Units 1,2 \& 3 |  | Total | Unit 1, 2 \& 3 Flow $=$ |  | 267.6 | cfs | 267.6 cfs (check) |  |
| Number of Screen Channels = | 2 |  |  |  |  |  |  |  |
| Channel Bottom Elevation $=$ | -20 |  | Low Tide |  |  |  | Low Tide |  |
|  |  |  | In-Channel | Velocity $=$ | 1.14 | fps | Through-Screen Velocity $=$ | 2.00 |
| Channel Width $=$ | 12.5 |  | $\mathrm{R}=$ |  |  |  |  |  |
|  |  |  |  |  | 5.3555 |  |  |  |
| Channel Depth $=$ | 25 |  |  |  |  |  |  |  |
| Water Depth in Channels (Low Tide) | 9.37 \% |  |  |  |  |  |  |  |
| Water Deph in Channels (High Tide) | 18.27 tt |  |  |  |  |  |  |  |
|  |  |  | $\begin{aligned} & \text { High Tide } \\ & \text { In-Channel } \end{aligned}$ | Velocity $=$ | 0.59 | fps | \|High Tide Through-Screen Velocity = | 1.03 |
| Channel for Unit 4 |  | Total | Unit 4 Flow $=$ |  | 223.0 | cfs | 223 cfs (check) |  |
| Number of Screen Channels = | 2 |  |  |  |  |  |  |  |
| Channel Bottom Elevation $=$ | -20 |  | $\begin{aligned} & \text { Low Tide } \\ & \text { In-Channel } \end{aligned}$ | Velocity $=$ | 1.55 | fps | Low Tide <br> Through-Screen Velocity $=$ | 2.72 |
| Channel Width $=$ | 11.25 |  |  |  |  |  |  |  |
|  |  |  | $\mathrm{R}=$ |  | 4.073220868 |  |  |  |
| Channel Depth $=$ | 25.75 |  |  |  |  |  |  |  |
| Water Depth in Channels (Low Tide) | 6.39 |  |  |  |  |  |  |  |
| Water Depth in Channels shigh Tide) | 15.29 |  |  |  |  |  |  |  |
|  |  | High Tide |  |  | \|high Tide |  |  |  |

Џn-Channel Velocity $=0.65 \quad$ ips $\quad$ Through-Screen Velocily $=$ 1.13)

OPERATIONAL CONDITION 2 - TOTAL INTAKE FLOW $=322.58$ MGD


OPERATIONAL CONDITION $3-$ TOTAL INTAKE FLOW $=328.33 \mathrm{MGD}$

| Unit 1, 2 or 3 (One Pump) $=$ | 34.56 MGD |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Unit 4 (One Pump) $=$ | 144.01 MGD | $\begin{array}{r} 53.52 \mathrm{cfs} \\ 223 \mathrm{cfs} \end{array}$ |  |  |  |  |  |  |
| Unit 5 (One Pump) = | 149.76 MGD | 231.92 cfs |  |  |  |  |  |  |
| Total Pump Flow = | 328.33 MGD | 508.44 cfs |  |  |  |  |  |  |
| Channels for Units 1,2 \& 3 |  | Total | Unit 1,2 or | low = | 53.5 | cfs | 53.52 cfs (check) |  |
| Number of Screen Channels = | 2 |  |  |  |  |  |  | , |
| Channel Bottom Elevation = | -20 |  | Low Tide In-Channel | Velocity $=$ | 0.69 | fps | Low Tide <br> Through-Screen Velocity $=$ | 1.20 |
| Channel Width $=$ | 12.5 |  |  |  |  |  |  |  |
| Channel Depth = | 25 |  |  | $\mathrm{R}=$ | 2.49 |  |  |  |

Water Depth in Channels (Low Tide) $\quad 3.12 \mathrm{ft}$
Water Deplh in Channels (High Tide) $\quad 12.02 \mathrm{ft}$

| Channel for Unit 4 |  |
| :--- | ---: |
| Number of Screen Channels = | 2 |
| Channel Bottom Elevation = | -20 |
| Channel Width = | 11.25 |
| Channel Depth = | 25.75 |
| Water Depth in Channels (Low Tide) | 6.39 |
| Water Depth in Channels (High Tide) | 15.29 |


| Channel for Unit 5 |  |
| :--- | ---: |
| Number of Screen Channels = | 3 |
| Channel Bottom Elevation = | -20 |
| Channel Width = | 11.25 |
| Channel Depth = | 27.75 |
| Water Depth in Channels (Low Tide) | 6.40 |
| Water Depih in Channels (High Tide) | 15.3 |


|  | High Tide In-Channe! | Velocity $=$ | 0.18 | fps | High Tide Through-Screen Velocity $=$ | 0.31 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Total | Unit 4 Flow = |  | 223.0 | cfs | 223 cfs (check) |  |
|  | $\begin{array}{\|l\|} \hline \text { Low Tide } \\ \text { In-Channe: } \end{array}$ | Velocity $=$ | 1.55 | fps | Low Tide <br> Through-Screen Velocity $=$ | 2.72 |
|  |  | $\mathrm{R}=$ | 4.07322 |  |  |  |
|  | $\begin{aligned} & \text { High Tide } \\ & \text { in-Channel } \end{aligned}$ | Velocity $=$ | 0.65 | fps | \|High Tide Through-Screen Velocity = | 1.13 |
| Total | Unit 5 Flow = |  | 231.74 | cfs | 231.92 cfs (check) |  |
|  | $\begin{aligned} & \text { Low Tide } \\ & \text { In-Channel } \\ & \hline \end{aligned}$ | Velocity $=$ | 1.07 | fps | Low Tide <br> Through-Screen Velocity $=$ | 1.57 |
|  |  | $R=$ | 4.64017 |  |  |  |
|  | High Tide <br> In-Channel | Velocity $=$ | 0.45 | fps | $\mid$ High Tide Through-Screen Velacity $=$ | 0.66 |

## OPERATIONAL CONDITION 5 - TOTAL INTAKE FLOW $=184.32$ MGD



Tide Level @ Screen Velocity of $0.5 \mathrm{fps}=\quad-0.687 \mathrm{ft}$

| Channel for Unit 5 <br> Number of Screen Channels $=$ | 3 | Total | Unit 5 Flow |  | 231.74 | cfs | 231.92 cfs (check) |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Channel Bottom Elevation $=$ | -20 |  | $\begin{array}{\|l\|} \hline \text { Low Tide } \\ \text { In-Channel } \end{array}$ | Velocity $=$ | 1.07 | fps | Low Tide Through-Screen Velocity = | 1.57 |
| Channel Widh $=$ | 11.25 |  |  |  |  |  |  |  |
| Channel Depth $=$ | 27.75 |  |  | $\mathrm{R}=$ | 4.64017 |  |  |  |
| Water Depth in Channels (Low Tlde) | 6.40 |  |  |  |  |  |  |  |
| Water Depth in Channels (High Tide) | 15.3 |  |  |  |  |  |  |  |
|  |  |  | High Tide In-Channe! | Velocity $=$ | 0.45 | fps | \|High Tide | 66 |

OPERATIONAL CONDITION $\mathbf{4}$ - TOTAL INTAKE FLOW $=\mathbf{2 1 8 . 8 8}$ MGD

| Unit 1,2 or 3 (Two Pumps) $=$ | 69.12 MGD | 107.04 cfs |
| :--- | ---: | :--- |
| Unit 5 (One Pump) $=$ | 149.66 MGD | 231.92 cf |
| Total Pump Flow $=$ | 218.88 MGD | 338.96 cfs |


| Channels for Units $1,2 \& 3$ <br> Number of Screen Channels $=$ | 2 |
| :--- | ---: |
| Channel Bottom Elevation = | -20 |
| Channel Width = | 12.5 |
| Channel Depth = | 25 |
| Water Deplh in Channels (Low Tlde) | 4.937 ft |

Total Unit 1, $2 \& 3$ Flow $=\quad 107.0 \quad \mathrm{cfs} \quad 107.04 \mathrm{cfs}$ (check)

| Low Tide <br> In-Channel | Velocity $=$ | 0.87 | fps | Low Tide <br> Throught-Screen Velocity $=$ |
| :--- | :--- | :--- | :--- | :--- |
|  | $\mathrm{R}=$ | 3.539169582 |  |  |


| Channel for Unit 5 |  |
| :--- | ---: |
| Number of Screen Channels = | 3 |
| Channel Bottom Elevation = | -20 |
| Channel Width = | 11.25 |
| Channel Depth = | 27.75 |
| Water Depth in Channels (Low Trde) | 6.40 |
| Water Depth in Channels (High Tide) | 15.3 |


| Unit 5 Flow $=$ |  | 231.74 | cfs | 231.92 cfs (check) |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
| Low Tide In-Channe | Velocity $=$ | 1.07 | fps | Low Tide Through-Screen Velocity $=$ | 1.57 |
|  | $\mathrm{R}=$ | 4.640171858 |  |  |  |
| High Tide |  |  |  | \|High Tide |  |
| In-Channel | Velocity $=$ | 0.45 | fps | Through-Screen Velocity = | 0.66 |

# Long-Term West Basin Water Level Analysis for Assessing Threshold Impingement Effects of Reduced Intake Flows at Agua Hedionda Lagoon 

Submitted by:
Scott A. Jenkins, Ph. D. and Joseph Wasyl
Dr. Scott A. Jenkins Consulting
14765 Kalapana Street, Poway, CA 92064
Submitted to:

Poseidon Resources, Suite 840
501 West Broadway
San Diego, CA 92101
21 January 2007

## 1) Introduction:

This study evaluates the long term water level variation in the West Basin of Agua Hedionda Lagoon. The objective of this analysis is to determine the persistence of water levels occurring higher than the threshold elevation for impingement losses during reduced flow rate operations of a stand alone desalination plant co-located at Encina Generating Station. There are two threshold water levels of interest for reduced flow operations ranging from 149.8 mgd to 304 mgd . These thresholds are -0.687 ft MSL and +4.83 ft MSL. The persistence analysis of these thresholds is performed by hydrodynamic model simulation of the water elevation history in the West Basin due to tidal forcing at the ocean inlet by historic ocean water levels measured at the nearby Scripps Pier tide gage (NOAA \# 931-0230) during the period of record 1980-2000. This time period was chosen because it coincides with the period of record used in the hydrodynamic studies in

Appendix E of the certified EIR (Jenkins and Wasyl, 2005). The verified ocean water level data on which this analysis is based was obtained from NOAA (2006).

Because of tidal muting by frictional losses through the ocean inlet of Agua Hedionda, it is not possible to use the Scripps Pier tide gage measurements directly to determine persistence analysis of. Such a simple approach would err on the side of over-estimating the percentage of time the water elevation in the West Basin of the lagoon met or exceeded the two threshold elevations of interest. Instead the tidal muting of the measured ocean water levels was determined through computer simulation of the lagoon tidal hydraulics. The TIDE_FEM tidal hydraulics model presented in Jenkins and Inman (1999) was gridded for a computational mesh of Agua Hedionda Lagoon as shown in Figure 1, using pre- and post dredging bathymetry from the 2002 dredge event from Jenkins and Wasyl (2003). The pre-dredging bathymetry featured the inlet bar in the west basin that was mapped during the October 2002 sounding shown in Figure 2. The postdredging survey performed in April 2003 indicated uniform deep water throughout the west basin with depths ranging from -20 ft NGVD to -30 ft NGVD, similar to that found in Figure 2-2 of Elwany, et al (2005). The lagoon model was excited at the ocean inlet by the ocean water level elevation time series measured by the Scripps Pier tide gage for the period 1980-2000. The simulated lagoon water levels in the west basin of Agua Hedionda were then sampled at 1 hour intervals, resulting in 183,432 separate outcomes of water elevation that could be subject to statistical analysis of persistence at or above the threshold elevations of interest.


Figure 1. Computational mesh for TIDE_FEM tidal hydraulics model of Agua Hedionda Lagoon.


Figure 2. Location key for 12 October 2002 bottom sediment sampling.

## 2) Results:

Time series of the simulated West Basin water levels for each from 1980 through 2000 are given in the upper panel of Figures A-1 through A-21 in Appendix-A. The lower panel of these Figures gives the west basin water level variation for the month containing the highest water level occurring that particular year. Figure 3 presents the probability density function (defined by red histogram bars) resulting from the 183,432 hourly realizations of West Basin water level. The blue curve in Figure 3 is the cumulative probability that the water level will be greater than or equal to a particular water level. The vertical dashed green line in Figure 3 defines the water elevation at -0.687 ft MSL, above which intake flow velocities at the Unit 1 intakes are below the impingement threshold. From the cumulative probability curve, we find that water elevations equal or exceed the -0.687 ft MSL threshold $67 \%$ of the time during this 21 year period of record. Thus it is more probable that impingement would not occur at the Unit 1 intakes. On the other hand, there was only one hourly outcome in the 21 year period of record when water elevations exceeded the Unit 5 threshold elevation at +4.83 (light blue dashed vertical line); and hence impingement would remain a definite possibility for nearly any tidal regime around the Unit 5 intake.

$a$
Figure 3. Probability density function and cumulative probability of the water level in the West Basin of Agua Hedionda Lagoon for the period of record 1980-2000.

## Reference:

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## APPENDIX-A: Time Series of West Basin Water Levels



Figure A-1. Water level in West Basin of Agua Hedionda Lagoon derived from TIDE FEM simulation using 1980 ocean water level measurements from Scripps Pier tide gauge (NOAA \# $\overline{9} 31-0230$ ).


Figure A-2. Water level in West Basin of Agua Hedionda Lagoon derived from TIDE FEM simulation using 1981 ocean water level measurements from Scripps Pier tide gauge (NOAA \#931-0230).


Figure A-3. Water level in West Basin of Agua Hedionda Lagoon derived from TIDE FEM simulation using 1982 ocean water level measurements from Scripps Pier tide gauge (NOAA \# 931-0230).


Figure A-4. Water level in West Basin of Agua Hedionda Lagoon derived from TIDE FEM simulation using 1983 ocean water level measurements from Scripps Pier tide gauge (NOAA \#931-0230).


Figure A-5. Water level in West Basin of Agua Hedionda Lagoon derived from TIDE FEM simulation using 1984 ocean water level measurements from Scripps Pier tide gauge (NOAA \# $\overline{9} 31-0230$ ).


Figure A-6. Water level in West Basin of Agua Hedionda Lagoon derived from TIDE FEM simulation using 1985 ocean water level measurements from Scripps Pier tide gauge (NOAA \# $931-0230$ ).


Figure A-7. Water level in West Basin of Agua Hedionda Lagoon derived from TIDE FEM simulation using 1986 ocean water level measurements from Scripps Pier tide gauge (NOAA \# $\overline{9} 31-0230$ ).



Figure A-8. Water level in West Basin of Agua Hedionda Lagoon derived from TIDE FEM simulation using 1987 ocean water level measurements from Scripps Pier tide gauge (NOAA \# $\overline{931-0230 \text { ). }}$


Figure A-9. Water level in West Basin of Agua Hedionda Lagoon derived from TIDE FEM simulation using 1988 ocean water level measurements from Scripps Pier tide gauge (NOAA \# $\overline{9} 31-0230$ ).


Figure A-10. Water level in West Basin of Agua Hedionda Lagoon derived from TIDE FEM simulation using 1989 ocean water level measurements from Scripps Pier tide gauge (NOAA \# 931-0230).


Figure A-11. Water level in West Basin of Agua Hedionda Lagoon derived from TIDE_FEM simulation using 1990 ocean water level measurements from Scripps Pier tide gauge (NOAA \# 931-0230).


Figure A-12. Water level in West Basin of Agua Hedionda Lagoon derived from TIDE FEM simulation using 1991 ocean water level measurements from Scripps Pier tide gauge (NOAA \# 931-0230).


Figure A-13. Water level in West Basin of Agua Hedionda Lagoon derived from TIDE FEM simulation using 1992 ocean water level measurements from Scripps Pier tide gauge (NOAA \#931-0230).


Figure A-14. Water level in West Basin of Agua Hedionda Lagoon derived from TIDE FEM simulation using 1993 ocean water level measurements from Scripps Pler tide gauge (NOAA \# 931-0230).


Figure A-15. Water level in West Basin of Agua Hedionda Lagoon derived from TIDE_FEM simulation using 1994 ocean water level measurements from Scripps Pler tide gauge (NOAA \#931-0230).


Figure A-16. Water level in West Basin of Agua Hedionda Lagoon derived from TIDE FEM simulation using 1995 ocean water level measurements from Scripps Pier tide gauge (NOAA \#931-0230).


Figure A-17. Water level in West Basin of Agua Hedionda Lagoon derived from TIDE_FEM simulation using 1996 ocean water level measurements from Scripps Pier tide gauge (NOAA \# 931-0230).


Figure A-18. Water level in West Basin of Agua Hedionda Lagoon derived from TIDE FEM simulation using 1997 ocean water level measurements from Scripps Pier tide gauge (NOAA \# 931-0230).


Figure A-19. Water level in West Basin of Agua Hedionda Lagoon derived from TIDE_FEM simulation using 1998 ocean water level measurements from Scripps Pier tide gauge (NOAA \# $9 \overline{3} 1-0230$ ).



Figure A-20. Water level in West Basin of Agua Hedionda Lagoon derived from TIDE FEM simulation using 1999 ocean water level measurements from Scripps Pier tide gauge (NOAA \#931-0230).


Figure A-21. Water level in West Basin of Agua Hedionda Lagoon derived from TIDE FEM simulation using 2000 ocean water level measurements from Scripps Pier tide gauge (NOAA \#931-0230).

