## RELATIONSHIPS BETWEEN FLOW FLUCTUATIONS AND REDD DEWATERING AND JUVENILE STRANDING FOR CHINOOK SALMON AND STEELHEAD IN THE SACRAMENTO RIVER BETWEEN KESWICK DAM AND BATTLE CREEK



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## DRAFT

Prepared by staff of The Energy Planning and Instream Flow Branch

# CVPIA INSTREAM FLOW INVESTIGATIONS SACRAMENTO RIVER BETWEEN KESWICK DAM TO BATTLE CREEK CHINOOK SALMON AND STEELHEAD REDD DEWATERING AND JUVENILE STRANDING 


#### Abstract

PREFACE

The following is the final report for the U. S. Fish and Wildlife Service's investigations on the effects of flow fluctuations on anadromous salmonid redd dewatering and juvenile stranding in the Sacramento River between Keswick Dam and Battle Creek. These investigations are part of the Central Valley Project Improvement Act (CVPIA) Instream Flow Investigations, a 7-year effort which began in February, 1995. Title 34, Section 3406(b)(1)(B) of the CVPIA, P.L. 102575 , requires the Secretary of the Interior to determine instream flow needs for anadromous fish for all Central Valley Project controlled streams and rivers, based on recommendations of the U. S. Fish and Wildlife Service after consultation with the California Department of Fish and Game (CDFG). The purpose of these investigations are to provide scientific information to the U. S. Fish and Wildlife Service Central Valley Project Improvement Act Program to assist in developing such recommendations for Central Valley rivers.


Written comments or questions about this report or these investigations should be submitted to:

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## INTRODUCTION

In response to substantial declines in anadromous fish populations, the Central Valley Project Improvement Act requires the doubling of the natural production of anadromous fish stocks, including the four races of chinook salmon (fall, late-fall, winter, and spring runs), steelhead, and white and green sturgeon. For the Sacramento River, the Central Valley Project Improvement Act Anadromous Restoration Plan calls for October through April flows ranging from 3,250 to $5,500 \mathrm{cfs}$, with the recommended flow varying with the October 1 carryover storage in Shasta Reservoir (U. S. Fish and Wildlife Service 1995). In December 1994, the U. S. Fish and Wildlife Service prepared a study proposal to identify the instream flow requirements for anadromous fish in certain streams within the Central Valley of California, including the Sacramento River. The purpose of this report is to model the effects of flow fluctuations on chinook salmon and steelhead redd dewatering and juvenile entrapment stranding in the Sacramento River between Keswick Reservoir and Battle Creek.

A 2-dimensional hydraulic and habitat model (RIVER2D) was used for the redd dewatering portion of this modeling, instead of the Physical Habitat Simulation ( $\mathrm{PHABSIM}^{1}$ ) component of the Instream Flow Incremental Methodology (IFIM). The 2-D model uses as inputs the bed topography and substrate of a site, total discharge at the upstream transect, and the water surface elevation at the downstream transect of the site, to predict the amount of habitat present in the site. The 2-D model avoids problems of transect placement, since the entire site can be modeled. The 2-D model also has the potential to model depths and velocities over a range of flows more accurately than PHABSIM because it takes into account upstream and downstream bed topography and bed roughness, and explicitly uses mechanistic processes (conservation of mass and momentum), rather than Manning's n and a velocity adjustment factor.

Other advantages of 2-D modeling are that it can explicitly handle complex habitats, including transverse flows, across-channel variation in water surface elevations, and flow contractions/expansions. The model scale is small enough to correspond to the scale of microhabitat use data with depths and velocities produced on a continuous basis, rather than in discrete cells. The 2-D model does a better job of representing patchy microhabitat features, such as gravel patches. The data can be collected with a stratified sampling scheme, with higher intensity sampling in areas with more complex or more quickly varying microhabitat features, and lower intensity sampling in areas with uniformly varying bed topography and uniform substrate. Bed topography and substrate mapping data can be collected at a very low flow, with the only data needed at high flow being water surface elevations at the upstream and downstream transects of the site and flow and edge velocities for validation purposes. In addition, alternative habitat suitability criteria, such as measures of habitat diversity, can be used.

[^0]The results of this study are intended to support or revise the flow recommendations above.

## METHODS

## Study Site Selection

We have divided the Sacramento River study area into six stream segments (Figure 1), based on hydrology and other factors: Grimes to Colusa (Segment 1); Deer Creek to Red Bluff Diversion Dam (Segment 2); above Lake Red Bluff to Battle Creek (Segment 3); Battle Creek to Cow Creek (Segment 4); Cow Creek to ACID (Segment 5); and ACID to Keswick Dam (Segment 6). Segment one addresses green and white sturgeon, while the other segments address chinook salmon and steelhead.

CDFG conducted mesohabitat mapping of the Sacramento River between Keswick Dam and Battle Creek. CDFG used thirteen mesohabitat types: bar complex glides, bar complex pools, bar complex riffles, bar complex runs, flatwater glides, flatwater pools, flatwater riffles, flatwater runs, side channel glides, side channel pools, side channel riffles, side channel runs, and offchannel areas (Snider et al 1992). The mesohabitat units (MHUs) were designated with numbers, starting with MHU \# 5 at Battle Creek to MHU \# 143 just below Keswick Dam.

The redd dewatering analysis was conducted using data from our eight spawning sites (Lower Lake Redding, Upper Lake Redding, Salt Creek, Bridge Riffle, Posse Grounds, Above Hawes Hole, Powerline Riffle and Price Riffle). Information on these sites is given in U.S. Fish and Wildlife Service 1999.

We surveyed both banks of the Sacramento River from Keswick Dam to Battle Creek to identify locations where juvenile chinook salmon could become trapped in inundated areas isolated from the main river channel when Sacramento River flows drop. Our surveys were conducted at relatively low flows (less than $8,000 \mathrm{cfs}$ ). The criteria that we used to identify stranding areas were: 1) the area would not drain to the main river channel; 2) the area would strand at river flows ranging from 3,250 to $15,000 \mathrm{cfs}$; and 3 ) the area was not the mouth of a tributary. We found 107 locations which would potentially become isolated from the main channel at flows ranging from 3,250 to $15,000 \mathrm{cfs}$. Twenty-seven of these sites were identified in October 1998. The remaining sites were identified in December 1999 and January and April 2000. The location of these sites are identified in Appendix A.

## Transect Placement (study site setup)

Details on transect placement for the spawning sites are given in U.S. Fish and Wildlife Service 1999. Details on site setup for our juvenile habitat modeling sites (used as discussed below for some of the stranding sites) are given in U.S. Fish and Wildlife Service 2005.

Figure 1
Sacramento River Stream Segments 2-6 ${ }^{2}$


$1 \mathrm{in}=7.2 \mathrm{mi}$
${ }^{2}$ Flows are the average flows for the period October 1974 to September 1993 at the top of each segment.

Three main approaches were used to determine the stranding flows ${ }^{3}$ for the 107 stranding sites: 1) for those stranding sites located in one of our juvenile habitat modeling sites, the 2-dimensional hydraulic model of the juvenile habitat site was used to determine the stranding flow for the stranding site; 2) for those stranding sites where the flow during our identification of the stranding site was at or slightly above or below the stranding flow for that site, we determined the stranding flow based on the flow on that date; and 3) for the remaining sites, we developed a stage-discharge relationship for the main river channel at the stranding site to determine the stranding flow. There were 10 stranding sites in our juvenile habitat modeling sites, 44 sites where the flow during our identification of the stranding site was at or slightly above or below the stranding flow for that site, and 53 sites for which we developed stage-discharge relationships. The first two categories of sites did not require any site setup or data collection, while the third category of site required the installation of a vertical benchmark (a lag bolt in a tree).

## Hydraulic and Structural Data Collection

Areas were determined for all of the stranding sites. For smaller sites, we determined the area by measuring the length and two to six widths of the stranding site, using an electronic distance meter; the area is calculated by multiplying the length times the average width. The areas of larger sites were measured on aerial photos or output of the RIVER2D modeling of our juvenile habitat modeling sites using a planimeter, or for Site 45B, using digitized aerial photos in GIS.

Vertical benchmarks were established at each of the 53 stranding sites for which we developed flow-habitat relationships to serve as the reference elevations to which all elevations (streambed and water surface) were tied. Vertical benchmarks consisted of lag bolts driven into trees.

Data required for developing a stage discharge relationship are: 1) water surface elevations (WSELs, stages), measured to the nearest 0.01 foot at three flows using standard surveying techniques (differential leveling); and 2) the stage of zero flow (SZF). Water surface elevations were measured at all but one of the 53 stranding sites for which we developed flow-habitat relationships at the following three flow ranges: $4,700-8,000 \mathrm{cfs}, 6,000-12,000 \mathrm{cfs}$ and 11,800 $15,100 \mathrm{cfs}$ (Appendix B) ${ }^{4}$. For one site, WSELs were measured at four different flows, with two

[^1]flows in the first flow range. We also measured the bed elevation of the stranding point (the lowest point at the connection between the stranding area and the main river channel) using differential leveling; the stage at the stranding flow was calculated by adding 0.1 feet to the bed elevation of the stranding point. After the stage discharge relationship was developed, it was used to determine what the flow is at the stranding flow stage. For most of the sites, the SZF was determined by making a traverse with a 600 kHz Broad-Band Acoustic Doppler Current Profiler (ADCP) across the main channel at the stranding point. For a few sites on side channels where the entire channel could be waded, the SZF was determined by measuring depths across the side channel with a wading rod. In both cases, the SZF was calculated as the difference between the WSEL on that date and the largest depth.

Flows for most sites were determined from gage data. There were 12 of the 53 stranding sites that were located on split channels. For 4 of these sites, we used flow/flow regressions between the split channel flow and the total Sacramento River discharge developed for the juvenile rearing habitat modeling sites. For the remaining 8 sites located on split channels, flows were measured when the WSELs were collected, to enable the development of flow/flow regressions between the split channel flow and the total Sacramento River discharge. For sites where the entire channel was wadable, flows were measured by making depth and velocity measurements by wading with a wading rod equipped with a Marsh-McBirney ${ }^{R}$ model 2000 or a Price AA velocity meter. For deeper sites, depth and velocity measurements in portions of the channel with depths greater than 3 feet were made with the ADCP , while depth and velocity measurements in shallower areas were made by wading with a wading rod equipped with a Marsh-McBirney ${ }^{\mathrm{R}}$ model 2000 or a Price AA velocity meter. The ADCP settings used are shown in Table 1. Starting at the water's edge, water depths and velocities were made at measured intervals using the wading rod and Marsh-McBirney ${ }^{R}$ model 2000 or Price AA velocity meter until the water became sufficiently deep to operate the ADCP (approximately 3 feet). The distance intervals of each depth and velocity measurement from the water's edge were measured using a hand held laser range finder. At the location of the last depth and velocity measurement made while wading, a buoy was placed to serve as a starting point for the ADCP. The boat was then positioned so that the ADCP started operation at the buoy, and water depth and velocity data were collected across the channel to the location near the opposite bank where water depths of approximately 3 feet were reached. A buoy was placed at the location where ADCP operation ceased and the procedure used for measuring depths and velocities in shallow water was repeated until the far bank water's edge was reached. Additional details on the ADCP operation are given in Gard and Ballard (2003).

Data collection started in October 1998 and was completed in August 2001.

Table 1
CFG Files ${ }^{5}$ Used for ADCP Data

|  |  |  |  |  |  |  |  |  |
| :--- | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| CFG File | Mode | Depth Cell <br> Size (ft) | Depth Cell <br> Number | Max Bottom <br> Track Depth <br> (ft) | Pings | WT $^{6}$ | First Depth <br> Cell (ft) | Blanking <br> Dist. (ft) |
| MD8A | 8 | 0.66 | 15 | 26 | 4 | 5 | 1.61 | 0.33 |
| MD4C | 4 | 0.33 | 30 | 26 | 4 | 5 | 1.51 | 0.33 |
| MD4E | 4 | 0.66 | 30 | 26 | 4 | 5 | 1.84 | 0.33 |
| MD4H | 4 | 0.66 | 100 | 52 | 4 | 5 | 1.84 | 0.33 |
| D45D | 8 | 0.66 | 30 | 26 | 4 | 5 | 1.94 | 0.66 |

Our eight spawning sites were originally modeled with PHABSIM (U.S. Fish and Wildlife Service 1999). Seven of these sites were subsequently modeled with RIVER2D for our juvenile habitat modeling (U.S. Fish and Wildlife Service 2005). We measured the horizontal location of the head and tail pins of the transects at the remaining spawning site, Bridge Riffle, with a total station, so that we could model this site with RIVER2D, using all of the points on these transects to determine the bed topography and substrate of this site. We collected 383 data points on the Bridge Riffle site transects, corresponding to a density of 3.9 points $/ 100 \mathrm{~m}^{2}$.

## Hydraulic Model Construction and Calibration

ASCII files of each ADCP traverse for flow or SZF measurements were produced using the Playback feature of the Transect program ${ }^{7}$. Each ASCII file was then imported into the Riverine Habitat Simulation (RHABSIM) ${ }^{8}$ Version 2.0 to produce the bed elevations, the component of the average water column velocities perpendicular to the transect, and stations (relative to the

[^2]${ }^{8}$ RHABSIM is a commercially-produced software (Payne and Associates 1998) that incorporates the modeling procedures used in PHABSIM.

[^3]start of the $A D C P$ traverse). RHABSIM was then used to output a second $A S C I$ file containing this data. For the SZF measurements, the second ASCI file was input into a QuattroPro spreadsheet where the maximum depth was subtracted from the measured WSEL to compute the SZF (Appendix B).

For the ADCP traverses made for flow measurements, the second ASCII file was input into a QuattroPro spreadsheet and combined with the velocity, depth, and station data collected in shallow water. We defined a statistic $(\mathrm{R})$ to provide a quality control check of the velocity measured by the $A D C P$ at a given station $n$, where $R={V e_{n}}_{n} /\left(\mathrm{Vel}_{n-1}+\mathrm{Vel}_{n+1}\right) / 2$ at station $n^{9} . R$ was calculated for each velocity where $\mathrm{Vel}_{n}, \mathrm{Vel}_{\mathrm{n}-1}$ and $\mathrm{Vel}_{\mathrm{n}+1}$ were all greater than $1 \mathrm{ft} / \mathrm{s}$ for each ADCP data set. Based on data collected using a Price AA velocity meter on the Lower American River, the acceptable range of $R$ was set at $0.5-1.6$. All verticals with $R$ values less than 0.5 or greater than 1.6 were deleted from each ADCP data set ${ }^{10}$. Discharges were calculated for each ADCP traverse, including the data collected in shallow water.

Flow/flow regressions were performed for sites which did not include the entire Sacramento River flow (Stranding Sites 14, 66, 87-90 and 97), using the flows measured in the site, and the corresponding total flows determined from gage readings ${ }^{11}$ (Table 2). For Stranding Sites 87-90 and 97, the regressions were developed from three sets of flows, with the entire river discharge around $6,000 \mathrm{cfs}, 9,000-10,000 \mathrm{cfs}$ and $15,000 \mathrm{cfs}$. For Stranding Site 66 , the regressions were only developed from two sets of flows, at 10,181 and $14,986 \mathrm{cfs}$, since the side channel on which this site was located stopped flowing when the total river flow dropped below 9,300 cfs. For Stranding Site 14, the regressions were also only developed from two sets of flows, at 6,152 and $8,826 \mathrm{cfs}$, made by wading the channel, since the channel could not be waded at flows above $9,000 \mathrm{cfs}$, and the channel was too shallow for accurate ADCP measurements. The site flows used in the regression were either the average of the ADCP traverses at the site or the flows measured with a wading rod and Price AA or Marsh-McBirney meter on the site. Calibration flows for Stranding Sites 14, 66, 87-90 and 97 (Table 4) were computed from the total discharge in Table 2 and the appropriate regression equation in Table 3.
${ }^{9} \mathrm{n}-1$ refers to the station immediately before station n and $\mathrm{n}+1$ refers to the station immediately after station n .
${ }^{10} \mathrm{We}$ also deleted velocities where $\mathrm{Vel}_{\mathrm{n}}$ was less than $1.00 \mathrm{ft} / \mathrm{s}$ and $\mathrm{Vel}_{\mathrm{n}-1}$ and $\mathrm{Vel}_{\mathrm{n}+1}$ were greater than $2.00 \mathrm{ft} / \mathrm{s}$, and where $\mathrm{Vel}_{\mathrm{n}}$ had one sign (negative or positive) and $\mathrm{Vel}_{\mathrm{n}-1}$ and $\mathrm{Vel}_{\mathrm{n}+1}$ had the opposite sign (when the absolute value of all three velocities were greater than $1.00 \mathrm{ft} / \mathrm{s}$ ); these criteria were also based on the Lower American River dataset (Gard and Ballard 2003).
${ }^{11}$ As shown in Table 2, the flow calculated at Bend Bridge from upstream and tributary gage readings often differed from the gage reading at Bend Bridge by less than $5 \%$ and never differed by more than $10 \%$. Flows could be calculated using either USBR or USGS flows measured at Keswick Dam; the flows selected for use were those which had the smaller Bend error.

Table 2
Sacramento River Flows at Stranding Study Sites ${ }^{12}$ (cfs)

| Date | Stranding Study Site Number |  |  |  |  |  |  |  | Bend error | Keswick <br> Flow Used |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | 1-8 | $\begin{gathered} 9-22 \& \\ 27-32 \end{gathered}$ | $\begin{gathered} 23-26,33- \\ 34,39-52, \\ 55-56 \end{gathered}$ | $\begin{gathered} 35-38,53- \\ 54,57 \\ 59-70 \end{gathered}$ | $\begin{gathered} 58,71- \\ 79 \mathrm{C}, 83- \\ 90 \end{gathered}$ | $\begin{gathered} 80-81, \\ 91,96- \\ 97 \end{gathered}$ | 92-94 | 82, 95 |  |  |
| 10/13/98 | 6580 |  |  |  |  |  |  |  | 7.64\% | USGS |
| 10/14/98 |  | 6152 | 6361 |  |  |  |  |  | 9.19\% | USGS |
| 5/25/99 | 10045 | 9813 | 9753 |  |  |  |  |  | 2.53\% | USBR |
| 8/5/99 | 12032 | 11822 | 11762 |  |  |  |  |  | 0.02\% | USBR |
| 12/9/99 |  | 7683 | 7683 | 7898 |  |  |  |  | 0.74\% | USBR |
| 12/10/99 |  |  | 7554 |  |  |  |  |  | 1.53\% | USBR |
| 1/12/00 |  | 4710 | 4710 | 4936 |  |  |  |  | 5.51\% | USGS |
| 3/17/00 |  | 11700 | 11700 | 12009 |  |  |  |  | 9.66\% | USGS |
| 4/25/00 |  |  |  | 8994 |  |  |  |  | 2.48\% | USGS |
| 4/26/00 |  |  |  | 8700 | 8926 | 9428 |  |  | 2.74\% | USGS |
| 4/27/00 |  |  | 8608 |  | 9035 |  | 9606 |  | 3.17\% | USGS |
| 7/10/00 |  | 14990 |  |  |  |  |  |  | 0.43\% | USGS |
| 7/11/00 |  | 14987 | 14927 | 14986 |  |  |  |  | 0.32\% | USGS |
| 7/12/00 |  |  |  |  | 14988 | 15409 |  |  | 0.43\% | USGS |
| 7/13/00 |  |  |  |  |  |  | 15071 |  | 0.79\% | USGS |
| 3/13/01 |  |  | 6150 |  |  |  |  |  | 5.55\% | USGS |
| 3/14/01 |  |  |  | 6086 | 6244 |  |  |  | 4.52\% | USGS |
| 3/15/01 |  |  |  |  | 5977 | 6454 | 6511 |  | 4.36\% | USGS |
| 7/9/01 |  |  |  |  | 14580 | . |  |  | 2.32\% | USGS |
| 8/20/01 |  |  |  |  | 10181 |  |  |  | 2.23\% | USGS |
| 8/22/01 |  |  |  |  |  | 9239 |  |  | 2.33\% | USGS |

See U.S. Fish and Wildlife Service 1999 and U.S. and Fish and Wildlife Service 2005, respectively, for details on the hydraulic model construction and calibration on the spawning site PHABSIM transects and our juvenile habitat modeling RIVER2D sites.

[^4]Table 3
Flow/Flow Regression Equations

| Stranding Study Site | Regression Equation | $\mathrm{R}^{2}$-value |
| :---: | :---: | :---: |
| Site 14 | Site $14 \mathrm{Q}=-377+0.1365 \times \mathrm{Q}$ | $1^{14}$ |
| Site 66 | Site $66 \mathrm{Q}=-542+0.0583 \times \mathrm{Q}$ | $1^{14}$ |
| Sites $87-90$ | Sites $87-90 \mathrm{Q}=-1976+0.3566 \times \mathrm{Q}$ | 0.9655 |
| Site 97 | Site $97 \mathrm{Q}=-1424+0.5696 \times \mathrm{Q}$ | 0.99999 |

Table 4
Calibration Flows for Stranding Study Sites 14, 66, 87-90 and 97 (cfs)

|  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: |
| Date | Site 14 | Site 66 | Sites $87-90$ | Site 97 |
| $10 / 14 / 98$ | 463 |  |  |  |
| $5 / 25 / 99$ | 963 |  |  |  |
| $8 / 5 / 99$ | 1237 |  |  |  |
| $7 / 11 / 00$ |  | 331 |  |  |
| $7 / 12 / 00$ |  | $\ddots$ | 3368 |  |
| $3 / 14 / 01$ |  |  | 201 |  |
| $3 / 15 / 01$ |  | 51 | 155 | 2252 |
| $8 / 20 / 01$ |  |  |  | 1654 |
| $8 / 22 / 01$ |  |  |  |  |

All stage-discharge data were compiled and checked before entry into PHABSIM data decks for the 53 stranding sites for which we developed flow-habitat relationships. A total of two to four sets of WSELs at widely spaced flows were used. Calibration flows in the data decks were the flows calculated from gage readings or the flows calculated from gage readings and the regression equations in Table 3. A separate deck was constructed for each set of study sites with the same calibration flows.
${ }^{13} \mathrm{Q}$ is the total river flow, Site 14 Q is the flow in Stranding Site 14 , etc.
14 Since only two flows were used in these regressions, the $\mathrm{R}^{2}$-values, by definition, were one.

[^5]The first step in the calibration procedure was to determine the best approach for WSEL simulation. Initially, the IFG4 hydraulic model (Milhous et al., 1989) was run on each deck to compare predicted and measured WSELs. This model produces a stage-discharge relationship using a log-log linear rating curve calculated from at least three sets of measurements taken at different flows. IFG4 is considered to have worked well if the following criteria are met: 1) the beta value (a measure of the change in channel roughness with changes in streamflow) is between 2.0 and $4.5 ; 2$ ) the mean error in calculated versus given discharges is less than $10 \% ; 3$ ) there is no more than a $25 \%$ difference for any calculated versus given discharge; and 4) there is no more than a 0.1 foot difference between measured and simulated WSELs ${ }^{15}$. For a majority of the sites, $I F G 4$ met the above criteria for $I F G 4$ (Appendix B).

For those transects/flow ranges modeled with $I F G 4$, the mean error and calculated-given discharge criteria were met in all cases, and the measured-simulated WSEL difference criterion for IFG4 was met in all cases except for stranding sites $15,16,30,39,46$ and 77 . We still used IFG4 for these sites because: 1) the difference between measured and simulated WSELs for all sites was less than 0.19 foot ${ }^{16} ; 2$ ) in all cases the stranding flow was not greater than the highest calibration flow; and 3) the calibration plots indicated that there was a linear log-log relationship over the range of calibration flows.

For stranding sites 57, 87, 88A and 90, the initial IFG4 calibration indicated that there was a significantly non-linear log-log relationship between stage and flow over the range of calibration flows. In these cases, we used a modification of $\operatorname{IFG4}$ where we only used two calibration flows. The calibration flows selected were those which bracketed the stranding flow. While this technique is not accepted for developing stage-discharge relationships, we concluded that it was sufficiently accurate for interpolating a stranding flow in between two calibration flows. Since only two flows are used in this method, the mean error and calculated versus given discharge criteria of IFG4 do not apply and the difference between measured and predicted WSELs will always be zero.

As shown in Appendix B, the beta coefficient values were less than 2.0 for stranding sites 15 , $57,72,74,81$ and 88 A . We concluded that this phenomenon were caused by channel characteristics which form hydraulic controls at some flows but not at others (compound controls), thus affecting upstream water elevations. Specifically, at lower flows the channel at these sites controlled the water surface elevations, while at higher flows the water surface elevations were controlled by downstream hydraulic controls. Accordingly, the performance of IFG4 for these sites was considered adequate despite the beta coefficient criterion not being met.

[^6]As shown in Appendix B, the beta coefficient values were greater than 4.5 for stranding sites 11, $13,37 / 38,69 / 70,87,88$ and 97 . We concluded that this phenomenon was caused by the presence of a downstream hydraulic control, such that the actual SZFs of these sites were greater than those in Appendix B. We determined that the correct SZF would have had a minimal effect on the estimated stranding flows for these sites - for example, a SZF which produced a beta coefficient of 4.5 for stranding site 38 would have only increased the stranding flow from 13,771 cfs to 13,775 cfs. As a result, we concluded that the SZFs in Appendix B were sufficiently accurate for the purposes of estimating stranding flows.

There were three other sites (stranding sites 36, 66 and 75 ) for which we developed stagedischarge relationships using methods other than IFG4. Sites 36 and 75 were located in bar complexes where there was a significant variation in WSEL across the entire river; IFG4 can not be used in this case since a basic assumption of IFG4 is that the WSEL does not vary across the channel. For site 36, we developed a stage-discharge relationship using a regression of $\log$ (WSEL) versus $\log$ (flow), with the three measured WSELs at 4,936, 6,086 and $12,009 \mathrm{cfs}$. There was a significantly non-log-linear relationship between stage and flow for site 75 over the range of measured WSELs; since the stranding flow for this site ended up being less than 3,250 cfs, we used the two lower measured WSELs to estimate the stranding flow. We used a regression equation of the form $\log ($ WSEL -A$)=\mathrm{B}+\mathrm{C} x \log$ (flow) for site 75 , where we determined $A$ in the field by subtracting the maximum depth in the main channel from the WSEL measured at the stranding location ${ }^{17} ; \mathrm{B}$ and C were derived from the regression. For site 66, we developed stage-discharge relationships using a regression of $\log$ (WSEL-SZF) versus $\log$ (flow), but only using two flows ( $14,986 \mathrm{cfs}$ and $10,181 \mathrm{cfs}$ ). As discussed above, we only measured WSELs at two flows for site 66. The regression equations for these sites are given in Table 5.

Table 5
Stage-Discharge Regression Equations

| Stranding Study Site | Regression Equation | $\mathrm{R}^{2}$-value |
| :---: | :---: | :---: |
| Site 36 | $\log (\mathrm{WSEL})=1.913+0.01979 \times \log (\mathrm{Q})$ | 0.9997 |
| Site 66 | $\log (\mathrm{WSEL}-96.0)=-0.506+0.376 \times \log ($ Site 66 Q$)$ | $1^{19}$ |
| Site 75 | $\log ($ WSEL -80.8$)=0.7670+0.0755 \times \log (\mathrm{Q})$ | $1^{19}$ |

[^7]The stranding flows for the 107 stranding sites are given in Appendix A. Sites 1-8, located upstream of ACID, had different stranding flows depending on whether the ACID dam was in or out. The stage-discharge relationships for 13 of the stranding sites resulted in a stranding flow of less than 3,250 cfs; these sites were thus dropped from consideration, since we are identifying areas that strand at flows between 3,250 and $15,000 \mathrm{cfs}$. Another two sites were dropped from consideration because they stranded at flows significantly greater than 15,000 cfs.

For the Bridge Riffle site, the PHABSIM transect data was used in QuattroPro to create the input files (bed and substrate) for the 2-D modeling program. An artificial extension one channel-width-long was added upstream of the top of the site to enable the flow to be distributed by the model when it reached the study area, thus minimizing boundary conditions influencing the flow distribution at the upstream transect and within the study site. The bed files contain the horizontal location (northing and easting), bed elevation and initial bed roughness value for each point, while the substrate files contain the horizontal location, bed elevation and substrate code for each point. The initial bed roughness value for each point was determined from the substrate codes for that point and the corresponding bed roughness values in Table 6. The bed roughness values in Table 6 were computed as five times the average particle size. The bed and substrate files were exported from QuattroPro as ASCII files.

A utility program, R2D_BED (Steffler 2001b), was used to define the study area boundary and to refine the raw topographical data TIN (triangulated irregular network) by defining breaklines ${ }^{20}$ following longitudinal features such as thalwegs, tops of bars and bottoms of banks. Breaklines were also added along lines of constant elevation. The bed topography of Bridge Riffle site is shown in Figure 2.

An additional utility program, R2D_MESH (Steffler 2001a), was used to define the inflow and outflow boundaries and create the finite element computational mesh for the River2D model. R2D_MESH uses the final bed files as an input. The first stage in creating the computational mesh was to define mesh breaklines ${ }^{21}$ which coincided with the final bed file breaklines. Additional mesh breaklines were then added between the initial mesh breaklines, and additional nodes were added as needed to improve the fit between the mesh and the final bed file and to improve the quality of the mesh, as measured by the Quality Index (QI) value. The QI is a
${ }^{20}$ Breaklines are a feature of the R2D_Bed program which force the TIN of the bed nodes to linearly interpolate bed elevation and bed roughness values between the nodes on each breakline and force the TIN to fall on the breaklines (Steffler 2001b).
${ }^{21}$ Mesh breaklines are a feature of the R2D_MESH program which force edges of the computation mesh elements to fall on the mesh breaklines and force the TIN of the computational mesh to linearly interpolate the bed elevation and bed roughness values of mesh nodes between the nodes at the end of each breakline segment (Steffler 2001a). A better fit between the bed and mesh TINs is achieved by having the mesh and bed breaklines coincide.

Table 6
Initial Bed Roughness Values

| Substrate Code | Type | Particle Size (inches) | Bed Roughness (m) |
| :---: | :---: | :---: | :---: |
| 0.1 | Sand/Silt | $<0.1$ | 0.05 |
| 1 | Small Gravel | $0.1-1$ | 0.1 |
| 1.2 | Medium Gravel | $1-2$ | 0.2 |
| 1.3 | Medium/Large Gravel | $1-3$ | 0.25 |
| 2.3 | Large Gravel | $2-3$ | 0.3 |
| 2.4 | Gravel/Cobble | $2-4$ | 0.4 |
| 3.4 | Small Cobble | $3-4$ | 0.45 |
| 3.5 | Small Cobble | $3-5$ | 0.5 |
| 4.6 | Medium Cobble | $4-6$ | 0.65 |
| 6.8 | Large Cobble | $6-8$ | 0.9 |
| 8 | Large Cobble | $8-10$ | 1.25 |
| 9 | Boulder/Bedrock | Large Cobble | $>12$ |
| 10 |  | $10-12$ | 0.05 |

measure of how much the least equilateral mesh element deviates from an equilateral triangle. An ideal mesh (all equilateral triangles) would have a QI of 1.0. A QI value of at least 0.2 is considered acceptable (Steffler 2001a). The mesh for the Bridge Riffle site, with 4819 nodes, had a QI value of 0.30 . The percentage of the original bed nodes for which the mesh differed by less than 0.1 foot $(0.03 \mathrm{~m})$ from the elevation of the original bed nodes was $87 \%$ for the Bridge Riffle site. In most cases, the areas of the mesh where there was greater than a 0.1 foot ( 0.03 m ) difference between the mesh and final bed file were in steep areas; in these areas, the mesh would be within 0.1 foot $(0.03 \mathrm{~m})$ vertically of the bed file within 1 foot $(0.3 \mathrm{~m})$ horizontally of the bed file location. Given that we had a 1 -foot ( 0.3 m ) horizontal level of accuracy, such areas would have an adequate fit of the mesh to the bed file. The final step with the R2D_MESH software was to generate the computational (cdg) file.

Figure 2
Bed Topography of Bridge Riffle Study Site


Units of Bed Elevation are meters.
The cdg file was opened in the RIVER2D software, where the computational bed topography mesh was used together with the WSEL at the bottom of the site, the flow entering the site, and the bed roughnesses of the computational mesh elements to compute the depths, velocities and WSELs throughout the site. The basis for the current form of RIVER2D is given in Ghanem et al (1995). The computational mesh was run in RIVER2D to steady state at a mid-range flow $(15,149 \mathrm{cfs})$ for which WSELs were measured, and the WSELs predicted by RIVER2D at the upstream end of the site were compared to the WSELs measured at both upstream transects. The bed roughnesses of the computational mesh elements were then modified by multiplying them by a constant bed roughness multiplier (BR Mult) until the WSELs predicted by RIVER2D at the upstream transect locations matched the WSELs measured at both upstream transects. The values of all other River2D hydraulic parameters were left at their default values (upwinding coefficient $=0.5$, minimum groundwater depth $=0.05 \mathrm{~m}$, groundwater transmissivity $=0.1$, groundwater storativity $=1$, and eddy viscosity parameters epsilon $1=0.01$, epsilon2 $=0.5$ and epsilon3 $=0.1$ ).

A stable solution will generally have a solution change ( $\operatorname{Sol} \Delta$ ) of less than 0.00001 and a net flow (Net Q) of less than 1\% (Steffler and Blackburn 2001). In addition, solutions for low gradient streams should usually have a maximum Froude Number (Max F) of less than one ${ }^{22}$. Finally, the WSEL predicted by the 2-D model should be within 0.10 foot ( 0.031 m ) of the

[^8]WSEL measured at the upstream transects ${ }^{23}$. The calibrated cdg file for Bridge Riffle site, with a BR Mult of 0.3 , had a Sol $\Delta$ of $2 \times 10^{-7}$, a net Q of $0.2 \%$, and a Max F of 1.00 . To get the Bridge Riffle site to calibrate, we needed to lower the downstream WSEL by 0.1 foot; the downstream WSELs for the simulation flows were also lowered by 0.1 foot. This allowed us to calibrate the model while still having the WSEL at the downstream transect within 0.1 foot of the measured WSEL. The Bridge Riffle site calibrated cdg file had WSELs that differed by more than 0.1 foot $(0.031 \mathrm{~m})$ from the measured WSELs for transects 2 and 3 main channel ${ }^{24}$ (Table 7). For transect 2 , the predicted WSELS near the water's edge, where the WSELs were measured, were within 0.1 foot ( 0.031 m ) of the measured WSELs. For much of the Sacramento River, the WSEL going across the river will differ by more than 0.1 foot ( 0.031 m ), with up to a 3 -foot ( 0.91 m ) measured difference in WSEL between the two banks in some areas, such as the Posse Grounds site. Accordingly, we conclude that the calibration for these transects was acceptable. For transect 3 main channel, the simulated WSEL on the left bank only differed by 0.18 foot $(0.054$ m ) from the measured WSEL, and this was only at a small area right at the water's edge. Given the above discussion, we conclude that the WSEL calibration of Bridge Riffle site was acceptable.

Table 7
2-D WSEL Calibration Statistics

|  | Difference (measured versus predicted WSELs, ft) ${ }^{25}$ |  |  |
| :---: | :---: | :---: | :---: |
| Transect | Average | Standard Deviation | Maximum |
| 1 | 0.10 | $\cdots$ | 0.10 |
| 2 | 0.03 | 0.28 | 0.49 |
| 2 LB | 0.07 | 0.01 | 0.08 |
| 3 SC | 0.07 | 0.002 | 0.08 |
| 3 MC | 0.30 | 0.15 | 0.49 |
| 3 MC LB | 0.09 | 0.04 | 0.18 |

[^9]Velocity validation is the final step in the preparation of the hydraulic models for use in habitat simulation. Velocities predicted by RIVER2D were compared with measured velocities to determine the accuracy of the model's predictions of mean water column velocities. The measured velocities used were the velocities measured on the three transects. See Appendix C for velocity validation statistics. Although there was a strong correlation between predicted and measured velocities, there were significant differences between individual measured and predicted velocities. In general, the simulated and measured velocities profiles at the three transects (Appendix $\mathrm{C}^{26}$ ) were relatively similar in shape. Differences in magnitude in most cases are likely due to: (1) operator error during data collection, i.e., the probe was not facing precisely into the direction of current; (2) range of natural velocity variation at each point over time resulting in some measured data points at the low or high end of the velocity range averaged in the model simulations; (3) aspects of the bed topography of the site that were not captured in our data collection; (4) the effect of the velocity distribution at the upstream boundary of the site ${ }^{27}$; and (5) the measured velocities being the component of the velocity in the downstream direction, while the velocities predicted by the 2-D model were the absolute magnitude of the velocity ${ }^{28}$. As shown in the figures in Appendix C, we attribute most of the differences between measured and predicted velocities to noise in the measured velocity measurements; specifically, for the transects, the simulated velocities typically fell within the range of the measured velocities of the two to three ADCP traverses made on each transect. The 2-D model integrates effects from the surrounding elements at each point. Thus, point measurements of velocity can differ from simulated values simply due to the local area integration that takes place. As a result, the area integration effect noted above will produce somewhat smoother lateral velocity profiles than the observations.

The flow and downstream WSEL in the calibrated Bridge Riffle site cdg file were changed to simulate the hydrodynamics of the sites at the simulation flows ( $3,250 \mathrm{cfs}$ to $5,500 \mathrm{cfs}$ by 250 cfs increments, $5,500 \mathrm{cfs}$ to $8,000 \mathrm{cfs}$ by 500 cfs increments, $8,000 \mathrm{cfs}$ to $15,000 \mathrm{cfs}$ by $1,000 \mathrm{cfs}$ increments, and $15,000 \mathrm{cfs}$ to $31,000 \mathrm{cfs}$ by $2,000 \mathrm{cfs}$ increments). The cdg file for each flow contained the WSEL predicted by PHABSIM at the downstream transect at that flow minus 0.1
${ }^{26}$ Velocities were plotted versus northing since the transects were orientated primarily north-south.

27 River2D distributes velocities across the upstream boundary in proportion to depth, so that the fastest velocities are at the thalweg. In contrast, the bed topography of a site may be such that the fastest measured velocities may be located in a different part of the channel. Since we did not measure the bed topography above a site, this may result in River2D improperly distributing the flow across the top of the site. As discussed above, we added artificial upstream extensions to the sites to try to address this issue.
${ }^{28}$ For areas with transverse flow, this would result in the 2-D model appearing to overpredict velocities even if it was actually accurately predicting the velocities.

[^10]foot. Each discharge was run in RIVER2D to steady state. Again, a stable solution will generally have a Sol $\Delta$ of less than 0.00001 and a Net Q of less than $1 \%$. In addition, solutions will usually have a Max F of less than one. The production cdg files all had a $\operatorname{Sol} \Delta$ of less than 0.00001 and a Net Q of less than 1\% (Table 8). The maximum Froude Number was greater than 1 for 16 simulated flows (Table 8); however, we considered these production runs to be acceptable since the Froude Number was only greater than 1 at a few nodes, with the vast majority of the area within the site having Froude Numbers less than 1. Also, these nodes were located either at water's edge or where water depth was extremely shallow, typically approaching zero and would be expected to have an insignificant effect on the model results.

## Habitat Suitability Criteria (HSC) Development

We assumed that there would be reduced survival of eggs or pre-emergent fry, and thus spawning habitat would be lost, if the tailspill was exposed or if velocities dropped to the point where there was insufficient intragravel flow through the redd. While we did not make measurements on tailspill depths for Sacramento River redds, we did take these measurements for 851 fall-run chinook salmon and 106 steelhead/rainbow trout redds on the Yuba River. There was a significant positive correlation between the depth of the redds and the difference between the redd depth and tailspill depth for both fall-run chinook salmon ( $\mathrm{R}^{2}=0.74, \mathrm{p}=2 \times 10^{-251}$ ) and steelhead/rainbow trout $\left(\mathrm{R}^{2}=0.04, \mathrm{p}=0.03\right)$ redds (Figures 3 and 4). When only redds with depths less than 2 feet ${ }^{29}$ were considered, the correlations for fall-run chinook salmon ( $\mathrm{n}=664$, $\mathrm{R}^{2}=0.31, \mathrm{p}=1 \times 10^{-55}$ ) and steelhead/rainbow trout $\left(\mathrm{n}=26, \mathrm{R}^{2}=0.39, \mathrm{p}=0.0005\right)$ were still significant. However, since we needed to pick a single value of the difference between the tailspill and redd depths for the redd dewatering analysis, we selected the average difference for fall-run chinook salmon ( 0.5 foot) and steelhead/rainbow trout ( 0.2 foot) redds with redd depths less than 2 feet. If the tailspill is 0.5 foot higher than the depth at the head of the pit (the depth used to compute spawning habitat), chinook salmon spawning habitat would be lost if the spawning depth fell below 0.5 foot. Similarly, if the tailspill is 0.2 foot higher than the depth at the head of the pit (the depth used to compute spawning habitat), steelhead spawning habitat would be lost if the spawning depth fell below 0.2 foot. We assumed that there would be insufficient intragravel flow through the redd if the spawning velocity was less than the lowest velocity at which we found a fall-run, late-fall-run or winter-run chinook salmon redd in the Sacramento River, or the lowest velocity of a steelhead redd in the Lower American River (the source of the steelhead spawning HSC used for the Sacramento River). The lowest velocities we found in measurements of Sacramento River fall-run, late-fall-run and winter-run chinook salmon were, respectively, $0.32 \mathrm{ft} / \mathrm{s}, 0.32 \mathrm{ft} / \mathrm{s}$ and $0.87 \mathrm{ft} / \mathrm{s}$ (U.S. Fish and Wildlife Service 2003). The lowest velocity of a steelhead redd in the Lower American River was $0.73 \mathrm{ft} / \mathrm{s}$ (U.S. Fish and Wildlife Service 1996a). The redd dewatering criteria used are given in Table 9.

[^11]Table 8
Bridge Riffle Site Simulation Statistics

| Flow (cfs) | Net Q | Sol $\mathbf{\Delta}$ | Max F |
| :---: | :---: | :---: | :---: |
| 3250 | $0.3 \%$ | .000004 | 1.34 |
| 3500 | $0.3 \%$ | .000008 | 1.35 |
| 3750 | $0.3 \%$ | .000007 | 1.36 |
| 4000 | $0.2 \%$ | .000004 | 1.38 |
| 4250 | $0.1 \%$ | .000005 | 1.41 |
| 4500 | $0.1 \%$ | .000005 | 1.46 |
| 4750 | $0.02 \%$ | $<.000001$ | 1.46 |
| 5000 | $0.04 \%$ | .000003 | 1.45 |
| 5250 | $0.1 \%$ | .000008 | 1.43 |
| 5500 | $0.1 \%$ | .000002 | 1.33 |
| 6000 | $0.01 \%$ | .000003 | 1.05 |
| 6500 | $0.04 \%$ | .000006 | 1.00 |
| 7000 | $0.2 \%$ | .000005 | 1.55 |
| 7500 | $0.2 \%$ | .000004 | 1.00 |
| 8000 | $0.3 \%$ | .000003 | 1.00 |
| 9000 | $0.7 \%$ | $<.000001$ | 1.00 |
| 10000 | $0.01 \%$ | $<.000001$ | 1.00 |
| 11000 | $0.1 \%$ | $<.000001$ | 1.03 |
| 12000 | $0.1 \%$ | $<.000001$ | 1.00 |
| 13000 | $0.1 \%$ | $<.000001$ | 1.00 |
| 14000 | $0.6 \%$ | .000003 | 1.00 |
| 15000 | $0.2 \%$ | $<.000001$ | 1.00 |
| 17000 | $0.4 \%$ | $<.000001$ | 1.00 |
| 19000 | $0.6 \%$ | $<.000001$ | 1.00 |
| 21000 | $0.8 \%$ | .000006 | 1.00 |
| 23000 | $0.5 \%$ | .000003 | 1.00 |
| 25000 | $0.04 \%$ | $<.000001$ | 1.38 |
| 27000 | $0.1 \%$ | $<.000001$ | 1.06 |
| 29000 | $0.05 \%$ | $<.000001$ | 1.00 |
| 31000 | $0.1 \%$ | $<.000001$ | 4.34 |
|  |  |  |  |
|  |  |  |  |
|  | 0 |  |  |

Figure 3
Tailspill and Redd Depths for Yuba River Fall-run Chinook Salmon Redds


Figure 4
Tailspill and Redd Depths for Yuba River Steelhead/Rainbow Trout Redds


Table 9
Redd Dewatering HSC

| Water |  | Water |  | Channel |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
| Velocity (ft/s) | SI Value | Depth (ft) | SI Value | Index Value | SI Value |
| Fall-run Chinook Salmon |  |  |  |  |  |
| 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 |
| 0.31 | 0.00 | 0.50 | 0.00 | 1.00 | 1.00 |
| 0.32 | 1.00 | 0.52 | 1.00 | 100.0 | 1.00 |
| 100.0 | 1.00 | 100.0 | 1.00 |  |  |
| Late-fall-run Chinook Salmon |  |  |  |  |  |
| 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 |
| 0.31 | 0.00 | 0.50 | 0.00 | 1.00 | 1.00 |
| 0.32 | 1.00 | 0.52 | 1.00 | 100.0 | 1.00 |
| 100.0 | 1.00 | 100.0 | 1.00 |  |  |
| Winter-run Chinook Salmon |  |  |  |  |  |
| 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 |
| 0.86 | 0.00 | 0.50 | 0.00 | 1.00 | 1.00 |
| 0.87 | 1.00 | 0.52 | 1.00 | 100.0 | 1.00 |
| 100.0 | 1.00 | 100.0 | 1.00 |  |  |
| Steelhead |  |  |  |  |  |
| 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 |
| 0.72 | 0.00 | 0.20 | 0.00 | . 1.00 | 1.00 |
| 0.73 | 1.00 | 0.23 | 1.00 | 100.0 | 1.00 |
| 100.0 | 1.00 | 100.0 | 1.00 |  |  |

## Habitat Simulation

We classified the stranding sites as either off-channel-areas or in-channel-areas (Appendix A). In our 1996 snorkel surveys of juvenile chinook salmon (U.S. Fish and Wildlife Service 1996b), we found an average of 6.3 fish $/ 1000 \mathrm{ft}^{2}$ in off-channel-areas, and an average of 30.2 fish $/ 1000 \mathrm{ft}^{2}$ in in-channel-areas. We multiplied the areas of each stranding site by the appropriate fish density to determine the average number of fish in each stranding site. These were summed by range of stranding flows to determine the total number of juvenile salmon stranded in the Sacramento River between Keswick Dam and Battle Creek for different drops in flow (Appendix D). For example, if the Sacramento River flow drops from 3,750 to $3,250 \mathrm{cfs}, 11,227$ fish will be stranded ${ }^{30}$. In contrast, if the Sacramento River flow drops from 15,000 to $4,000 \mathrm{cfs}$ with the ACID dam boards in, 12,044 fish will be stranded. There are minor differences in the number of fish stranded with the ACDD dam boards out versus in due to the different stranding flows for stranding sites 1 to 8 when the ACID dam boards are out versus in.

We conducted an effective spawning analysis with River2D to determine the percentage loss of fall-run, late-fall-run and winter-run chinook salmon and steelhead spawning habitat in the Sacramento River between Keswick Dam and Battle Creek associated with drops in flow. An effective spawning analysis examines on a node by node basis the depths and velocities at lower flows, and sets the weighted useable area represented by each node at a given flow to zero if the depth or velocity at a lower flow are less than the parameters in Table 9; if the depth and velocity at the lower flow are both greater than the parameters in Table 9, the weighted useable area represented by a given node is not changed. By adding up the resulting weighted useable areas represented by all the nodes, the effective spawning analysis computes how much weighted useable area remains after the flow drops. The percentage loss in spawning habitat is then computed as:
(original spawning habitat - remaining spawning habitat)/original spawning habitat
We conducted the effective spawning habitat analysis by producing an output file containing the spawning combined habitat suitability from River2D with the spawning flow file for a given site. This file was then used as a channel index file for the River2D files for the dewatering flows for that site, along with the HSC in Table 9, to compute the remaining spawning habitat.

A byproduct of the effective spawning analysis were new flow-habitat relationships for fall-run, late-fall-run and winter-run chinook salmon and steelhead spawning for the eight spawning sites (Appendix F) and the three segments between Keswick Dam and Battle Creek (Appendix G) computed using River2D.

[^12]
## RESULTS

The effects of flow drops on juvenile salmon entrapment stranding are shown in Figures 5 and 6 and Appendix D. The results indicate that, as expected, greater drops in flow are associated with increased numbers of stranded juvenile salmon, but that substantial juvenile stranding could be avoided by keeping flows above $3,750 \mathrm{cfs}$. These results could be used to determine the amount of take of juvenile salmon associated with a given drop in releases of flow from Keswick Dam. The effects of flow drops on redd dewatering are shown in Figures 7 to 14 and Appendix E. Similar to juvenile stranding, the results of the redd dewatering analysis indicate that, as expected, greater drops in flow result in a greater percentage of salmon and steelhead redds being dewatered. These results also suggest that the percentage of redds dewatered associated with a given drop of flow is less with the ACID Dam in, versus the ACID Dam out, reflecting the deeper water conditions above the ACID Dam when the dam is in versus when the dam is out. These results could be used to determine the amount of take of chinook salmon and steelhead eggs and pre-emergent fry associated with a given drop in releases of flow from Keswick Dam.

Flow-habitat relationships for fall-run, late-fall-run and winter-run chinook salmon and steelhead spawning, calculated using River2D, are given in Appendices F and G. We recommend that these results be used, rather than the results in U.S. Fish and Wildlife Service 2003, because of the improved prediction of flow-habitat relationships with River2D, versus PHABSIM.

Figure 5
Stranding of Juvenile Chinook Salmon with ACD Dam out


USFWS, SFWO, Energy Planning and Instream Flow Branch
Sacramento River (Keswick Dam to Battle Creek) Redd Dewatering and Juvenile Stranding Final Report

Figure 6
Stranding of Juvenile Chinook Salmon with ACID Dam in


USFWS, SFWO, Energy Planning and Instream Flow Branch
Sacramento River (Keswick Dam to Battle Creek) Redd Dewatering and Juvenile Stranding Final Report June 22, 2006

Figure 7
Dewatering of Fall-run Chinook Salmon Redds with ACID Dam out


USFWS, SFWO, Energy Planning and Instream Flow Branch
Sacramento River (Keswick Dam to Battle Creek) Redd Dewatering and Juvenile Stranding Final Report
June 22, 2006

Figure 8
Dewatering of Fall-run Chinook Salmon Redds with ACDD Dam in


USFWS, SFWO, Energy Planning and Instream Flow Branch
Sacramento River (Keswick Dam to Battle Creek) Redd Dewatering and Juvenile Stranding Final Report

Figure 9
Dewatering of Late-Fall-run Chinook Salmon Redds with ACID Dam out


USFWS, SFWO, Energy Planming and Instream Flow Branch
Sacramento River (Keswick Dam to Battle Creek) Redd Dewatering and Juvenile Stranding Final Report
June 22, 2006

Figure 10
Dewatering of Late-Fall-run Chinook Salmon Redds with ACDD Dam in .


USFWS, SFWO, Energy Planning and Instream Flow Branch
Sacramento River (Keswick Dam to Battle Creek) Redd Dewatering and Juvenile Stranding Final Report
June 22, 2006

Figure 11
Dewatering of Winter-run Chinook Salmon Redds with ACID Dam out


USFWS, SFWO, Energy Planning and Instream Flow Branch
Sacramento River (Keswick Dam to Battle Creek) Redd Dewatering and Juvenile Stranding Final Report

Figure 12
Dewatering of Winter-run Chinook Salmon Redds with ACID Dam in


Figure 13
Dewatering of Steelhead Redds with ACID Dam out


USFWS, SFWO, Energy Planning and Instream Flow Branch
Sacramento River (Keswick Dam to Battle Creek) Redd Dewatering and Juvenile Stranding Final Report

Figure 14
Dewatering of Steelhead Redds with ACID Dam in


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## APPENDIX A <br> STRANDING SITE LOCATIONS

| Stranding Site \# | River Mile | River Bank | MHU \# | Stranding Flow (cfs) ${ }^{31}$ | Stranding Area ( $\mathrm{ft}^{2}$ ) | Type |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1 | 298.8 | Left | 139 | 21,250/5,000 | 19,579 | In |
| 3A | 300.6 | Left | 143 | 12,750/11,100 | 684 | OCA |
| 3B | 300.6 | Left | 143 | 5,200/4,625 | 2,673 | OCA |
| 4 | 300.8 | Left | 143 | 7,400/6,580 | 4,838 | OCA |
| 5 | 301.4 | Left | 143 | 20,000/4,825 | 2,107 | OCA |
| 6 | 302.0 | Right | 143 | 8,128 | 1,200 | In |
| 7 | 300.2 | Right | 141 | 5,250/<3,250 | 2,850 | In |
| 8 | 299.9 | Right | 140 | 8,200/5,100 | 12,906 | In |
| 9 | 292.5 | Left | 100 | 6,409 | 1,319 | OCA |
| 10 | 294.0 | Left | 109 | 5,950 | 600 | OCA |
| 11 | 295.2 | Left | 113 | <3,250 | --- | OCA |
| 12 | 295.2 | Left | 113 | <3,250 | 8,303 | OCA |
| 13 | 296.4 | Left | 129 | 4,500 | 1,056 | OCA |
| 14 | 296.5 | Left | 127 | 4,555 | 200,000 | OCA |
| 15 | 297.0 | Left | 127 | <3,250 | 5,373 | OCA |
| 16 | 297.4 | Left | 133 | <3,250 | .75,024 | In |
| 17 | 296.9 | Right | 132 | 4,844 | 1,296 | OCA |
| 18 | 296.7 | Right | 130 | 9,376 | 387 | OCA |
| 19 | 296.3 | Right | 123 | 5,950 | 3,164 | In |
| 20 | 295.5 | N/A | 114 | 9,337 | 13,640 | OCA |
| 21 | 295.3 | N/A | 114 | 6,050 | 47,611 | OCA |
| 22 | 294.9 | Right | 111 | <3,250 | 594 | OCA |
| 23 | 291.7 | Right | 96 | 4,360 | 4,497 | In |
| 24 | 291.8 | Left | 97 | 6,032 | 2,640 | In |
| 25 | 291.8 | Right | 97 | 4,248 | 5,612 | In |

${ }^{31}$ Sites 1 to 5, 7 and 8 are located above ACID and have a different stranding flow depending on whether the boards are in or out at ACID. The first flow is the stranding flow with boards out, while the second flow is the stranding flow with boards out.

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| Stranding Site \# | River Mile | River Bank | MHU \# | Stranding. Flow (cfs) | Stranding Area ( $\mathrm{ft}^{2}$ ) | Type |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 26 | 289.5 | Right | 80 | 4,849 | 423 | OCA |
| 27 | 293.7 | N/A | 107 | 3,946 | 106,000 | OCA |
| 28 | 293.7 | Right | 109 | <3,250 | 1,352 | OCA |
| 29 | 293.7 | Right | 108 | 7,483 | 300 | OCA |
| 30 | 293.1 | Right | 104 | 5,921 | 26,978 | In |
| 31 | 292.8 | Right | $\cdot 104$ | 14,276 | 580 | OCA |
| 32 | 292.8 | Right | 104 | 7,683 | 26,371 | In |
| 33 | 291.5 | Right | 91 | 14,927 | 21,500 | OCA |
| 34 | 290.3 | Right | 85 | 5,934 | 11,606 | OCA |
| 35 | 289.3 | Middle | 75 | 7,898 | 4,397 | In |
| 36 | 289.3 | Middle | 75 | 3,450 | 36,320 | In |
| 37 | 288.5 | Right | 67 | $<3,250$ | 4,700 | OCA |
| 38 | 288.5 | Right | 67 | 13,771 | 429 | OCA |
| 39A | 291.7 | Left | 98 | 4,752 | 4,118 | OCA |
| 39B | 291.7 | Left | 98 | 10,508 | 533 | OCA |
| 40 | 291.4 | Left | 91 | 10,747 | 13,739 | OCA |
| 41 | 290.3 | Left | 85 | 7,330 | 5,921 | OCA |
| 41A | 290.3 | Left | 85 | 4,640 | 3,233 | In |
| 42 | 290.3 | Left | 85 | $7,683>Q>4,710$ | 3,050 | OCA |
| 43 | 290.3 | Left | 85 | 4,440 | 9,020 | OCA |
| 44 | 290.0 | Left | 85 | 9,514 | 18,631 | OCA |
| 45A | 290.0 | N/A | 84 | <3,250 | 2,649 | In |
| 45B | 290.0 | N/A | 84 | 3,502 | 87,352 | In |
| 46 | 289.8 | N/A | 83 | 4,108 | 34,126 | In |
| 47 | 289.5 | Left | 81 | 9,661 | 432 | OCA |
| 48 | 289.4 | Left | 75 | 8,277 | 333 | In |
| 49 | 289.8 | Left | 83 | 4,640 | 5,066 | In |
| 50 | 289.6 | N/A | 82 | 4,440 | 40,594 | In |


| Stranding Site \# | River Mile | River Bank | MHU \# | Stranding Flow (cfs) | Stranding Area ( $\mathrm{ft}^{2}$ ) | Type |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 51 | 289.5 | N/A | $\begin{gathered} 78,80 \\ 82 \end{gathered}$ | 3,502 | 222,752 | In |
| 51A | 289.5 | N/A | 79 | 3,502 | 64,688 | OCA |
| 52 | 289.4 | N/A | 76 | 6,180 | 3,827 | In |
| 53 | 289.4 | N/A | 76 | 4,666 | 17,375 | In |
| 54 | 289.4 | N/A | 76 | 4,766 | 4,261 | In |
| 55 | 289.8 | Right | 84 | 14,727 | 3,630 | OCA |
| 56 | 289.7 | Right | 84 | 4,440 | 2,088 | In |
| 57 | 285.2 | Left | 46 | 5,265 | 713 | In |
| 58 | 283.3 | Left | 45 | <3,250 | 771 | OCA |
| 59 | 284.9 | Left | 46 | 6,086 | 760 | In |
| 60 | 287.7 | N/A | 61 | <3,250 | --- | In |
| 60 A | 287.7 | Right | 61 | 8,762 | 1,330 | In |
| 60B | 287.7 | Right | 61 | 8,962 | 1,170 | In |
| 61 | 287.9 | Left | 63 | 5,752 | 30,437 | In |
| 61 A | 287.9 | Left | 63 | 3,568 | 11,727 | In |
| 61B | 287.9 | Left | 63 | 6,286 | 624 | In |
| 62 | 287.8 | N/A | 61 | <3,250 | --- | In |
| 63 | 287.9 | Right | 64 | 8,762 | 480 | In |
| 64 | 287.6 | Right | 59 | 8,562 | 583 | OCA |
| 65 | 287.5 | Right | 60 | 8,762 | 943 | OCA |
| 66 | 286.3 | Right | 53 | 10,859 | 3,049 | OCA |
| 67 | 286.3 | Right | 53 | 5,986 | 924 | In |
| 68 | 285.4 | Right | 48 | 5,460 | 84,638 | OCA |
| 69 | 285.2 | Right | 47 | 4,450 | 2,345 | OCA |
| 70 | 285.2 | Right | 47 | 5,100 | 2,669 | OCA |
| 71 | 284.3 | Right | 45 | 3,664 | 493 | OCA |
| 72 | 283.6 | Right | 45 | 12,643 | 722 | OCA |


| Stranding Site \# | River Mile | River Bank | MHU \# | Stranding Flow (cfs) | Stranding Area ( $\mathrm{ft}^{2}$ ) | Type |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 73 | 282.8 | Right | 43 | 5,750 | 364 | OCA |
| 74 | 282.6 | Right | 42 | 4,591 | 235 | OCA |
| 75 | 281.3 | Right | 36 | $<3,250$ | 42,066 | OCA |
| 76 | 281.3 | Right | 36 | 8,826 | 5,918 | In |
| 77 | 281.0 | Right | 34 | 6,744 | 2,341 | OCA |
| 78 | 280.6 | Right | 33 | 6,672 | 2,331 | OCA |
| 798 | 280.6 | Right | 33 | 8,364 | 120 | OCA |
| 79 C | 280.6 | Right | 33 | 8,926 | 1,691 | OCA |
| 79A | 280.4 | Left | 31 | 8,926 | 693 | In |
| 80 | 279.9 | Right | 28 | 9,430 | 459 | In |
| 81 | 279.1 | Right | 26 | 13,546 | 1,814 | OCA |
| 82 | 273.0 | Right | 9 | 18,799 | 702 | OCA |
| 83 | 283.1 | Left | 44 | <3250 | 675 | OCA |
| 84 | 282.7 | Left | 43 | 9,100 | 3451 | OCA |
| 85 | 282.6 | Left | 41 | 28,422 | 7,097 | OCA |
| 86 | 280.8 | Right | 34 | 6,542 | 2,153 | In |
| 87 | 280.4 | Right | 30 | 6,417 | 2,129 | In |
| 88 | 280.3 | Right | 30 | 8,287 | 1,746 | In |
| 88A | 280.3 | Right | 30 | 11,496 | 1,089 | In |
| 89 | 280.3 | Right | 30 | 7,937 | 50 | OCA |
| 90 | 280.2 | Left | 30 | 5,674 | 650 | OCA |
| 91 | 278.5 | Left | 20 | 9,333 | 3,683 | OCA |
| 92 | 276.9 | Left | 14 | 8,333 | 1,871 | OCA |
| 93 | 275.6 | Left | 12 | 15,071 | 738 | OCA |
| 94 | 275.6 | Left | 12 | 11,083 | 675 | OCA |
| 95 | 271.7 | Right | 6 | 5,542 | 27,003 | In |
| 96 | 287.6 | Right | 21 | 9,406 | 1,159 | In |
| 97 | 287.6 | Right | 21 | 9,568 | 564 | In |

## APPENDIX $B$ <br> PHABSIM WSEL CALIBRATION

## Stage of Zero Flow Values

| Stranding Site | SZF |
| :---: | :---: |
| 6 | 61.3 |
| 9 | 85.2 |
| 11 | 85.7 |
| 12 | 89.5 |
| 13 | 80.8 |
| 14 | 92.7 |
| 15 | 95.8 |
| 16 | 81.6 |
| 17 | 87.7 |
| 20 | 86.8 |
| 22 | 89.2 |
| 24, 25 | 92.8 |
| 27, 28 | 89.4 |
| 30 | 89.1 |
| 31 | 86.8 |
| 34 | 85.3 |
| 37,38 | 76.5 |
| 39 | 92.1 |
| 40 | 93.0 |
| 44, 45 | 88.5 |
| 46 | 91.8 |
| 48 | 87.1 |
| 57 | 79.8 |
| 58 | 86.6 |


| Stranding Site | SZF |
| :---: | :---: |
| 61 | 90.3 |
| 66 | 96.0 |
| 68 | 88.1 |
| 69,70 | 78.0 |
| 71 | 88.7 |
| 72 | 87.3 |
| 74 | 89.3 |
| 77 | 88.5 |
| $78,79 \mathrm{~B}$ | 80.2 |
| 81 | 90.7 |
| 85 | 90.4 |
| 86 | 87.0 |
| 87 | 95.0 |
| 88 | 94.3 |
| 98 A | 95.8 |
| 89 | 93.6 |
| 90 | 90.5 |
| 97 | 83.8 |
| 98.7 |  |
|  |  |
| 97 |  |


| SITE | $\begin{gathered} \text { BETA } \\ \text { COEFF. } \end{gathered}$ | \%MEAN ERROR | Calculated vs. Given Disch. (\%) |  |  | Difference (measured vs. pred. WSELs) |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  | 6580 cfs | 10045 cfs | 12032 cfs | 6580 cfs | 10045 cfs | $\underline{12032 \mathrm{cfs}}$ |
| 6 | 3.88 | 0.3 | 0.2 | 0.5 | 0.4 | 0.01 | 0.04 | 0.03 |
|  | BETA | \%MEAN | Calculated vs. Given Disch. (\%) |  |  | Difference (measured vs. pred. WSELs) |  |  |
| SITE | COEFF. | ERROR | 6152 cfs | 9813 cfs | 11822 cfs | 6152 cfs | 9813 cfs | $\underline{1822 \mathrm{cfs}}$ |
| 9 | 3.78 | 0.1 | 0.0 | 0.2 | 0.1 | 0.00 | 0.01 | 0.00 |
| 11 | 5.56 | 0.6 | 0.3 | 0.9 | 0.4 | 0.01 | 0.02 | 0.01 |
| 12 | 3.79 | 0.4 | 0.2 | 0.6 | 0.5 | 0.00 | 0.01 | 0.01 |
| 13 | 7.05 | 0.4 | 0.2 | 0.6 | 0.4 | 0.00 | 0.01 | 0.01 |
| 16 | 4.02 | 3.4 | 2.1 | 5.3 | 3.0 | 0.07 | 0.18 | 0.11 |
| 17 | 2.91 | 0.3 | 0.1 | 0.5 | 0.3 | 0.00 | 0.02 | 0.01 |
| 20 | 3.01 | 0.0 | 0.0 | 0.0 | 0.0 | 0.00 | 0.00 | 0.00 |
| 22 | 2.73 | 1.0 | 0.5 | 1.5 | 1.0 | 0.01 | 0.05 | 0.04 |

BETA \%MEAN Calculated vs. Given Disch. (\%) Difference (measured vs. pred. WSELs) SITE COEFF ERROR $6 \underline{6152 \mathrm{cfs}} \quad \underline{9812 \mathrm{cfs}} 11822 \mathrm{cfs} \quad 6152 \mathrm{cfs} \quad 9812 \mathrm{cfs} \quad 11822 \mathrm{cfs}$

| 14 | 2.72 | 1.9 | 0.9 | 3.0 | 2.0 | 0.01 | 0.05 | 0.04 |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- |

BETA \%MEAN Calculated vs. Given Disch. (\%) Difference (measured vs. pred. WSELs)
SITE COEFF. ERROR $\quad 7910 \mathrm{cfs} \quad 11700 \mathrm{cfs} 14990 \mathrm{cfs} \quad 7910 \mathrm{cfs} \quad 11700 \mathrm{cfs} \quad 14990 \mathrm{cfs}$

| 15 | 1.96 | 3.2 | 2.2 | 4.9 | 2.6 | 0.04 | 0.11 | 0.07 |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- |


|  | BETA | \%MEAN | Calculated vs. Given Disch. (\%) |  |  | Difference (measured vs. pred. WSELs) |  |  |
| :--- | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| SITE | COEFF. | ERROR | $\underline{6092 \mathrm{cfs}}$ | $\underline{9808 \mathrm{cfs}}$ | $\underline{2418 \mathrm{cfs}}$ | $\underline{6092 \mathrm{cfs}}$ | $\underline{9808 \mathrm{cfs}}$ | $\underline{2418 \mathrm{cfs}}$ |
| 24,25 | 2.07 | 0.9 | 0.3 | 1.3 | 1.0 | 0.00 | 0.02 | 0.02 |

BETA \%MEAN Calculated vs. Given Disch. (\%) Difference (measured vs. pred. WSELs)

| SITE | COEFF. | ERROR | 7683 cfs | $\underline{11700 \mathrm{cfs}}$ | 14998 cfs | 7683 cfs | 11700 cfs | 14998 cfs |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 27, 28 | 3.36 | 0.5 | 0.3 | 0.8 | 0.5 | 0.01 | 0.02 | 0.01 |

[^14]

[^15]| SITE | $\begin{gathered} \text { BETA } \\ \text { COEFF. } \end{gathered}$ | \%MEAN <br> ERROR | Calculated vs. Given Disch. (\%) |  |  | Difference (measured vs. pred. WSELs) |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  | 6244 cfs | 8994 cfs | 14988 cfs | 6244 cfs | 8994 cfs | 14988 cfs |
| 58 | 3.24 | 2.0 | 1.9 | 3.1 | 1.1 | 0.05 | 0.09 | 0.04 |
|  | BETA | \%MEAN | Calculated vs. Given Disch. (\%) |  |  | Difference (measured vs. pred. WSELs) |  |  |
| SITE | COEFF. | ERROR | 6106 cfs | 8762 cfs | 14986 cfs | 6106 cfs | 8762 cfs | 14986 cfs |
| 61, 61A | 4.04 | 1.0 | 0.9 | 1.5 | 0.6 | 0.01 | 0.02 | 0.01 |
|  | BETA | \%MEAN | Calculated vs. Given Disch. (\%) |  |  | Difference (measured vs. pred. WSELs) |  |  |
| SITE | COEFF. | ERROR | 6086 cfs | 8700 cfs | 14986 cfs | 6086 cfs | 8700 cfs | 14986 cfs |
| 68 | 2.87 | 0.5 | 0.5 | 0.8 | 0.3 | 0.01 | 0.03 | 0.01 |
| 69, 70 | 6.17 | 2.0 | 1.9 | 3.0 | 1.0 | 0.06 | 0.10 | 0.04 |
|  | BETA | \%MEAN | Calculated vs. Given Disch. (\%) |  |  | Difference (measured vs. pred. WSELs) |  |  |
| SITE | COEFF. | ERROR | $\underline{6244 \mathrm{cfs}}$ | 8926 cfs | 14986 cfs | 6244 cfs | 8926 cfs | 14986 cfs |
| 71 | 3.01 | 2.1 | 2.0 | 3.2 | 1.1 | 0.05 | 0.09 | 0.04 |
|  | BETA | \%MEAN | Calculated vs. Given Disch. (\%) |  |  | Difference (measured vs. pred. WSELs) |  |  |
| SITE | COEFF. | ERROR | 6244 cfs | 8926 cfs | 14988 cfs | 6244 cfs | 8926 cfs | 14988 cfs |
| 72 | 1.88 | 2.2 | 2.2 | 3.4 | 1.2 | 0.04 | 0.09 | 0.04 |
| 74 | 1.83 | 1.1 | 1.1 | 1.7 | 0.7 | 0.03 | 0.05 | 0.03 |
| 77 | 2.42 | 2.9 | 2.9 | 4.4 | 1.4 | 0.09 | 0.13 | 0.06 |
| 78, 79B | 4.33 | 0.4 | 0.4 | 0.7 | 0.3 | 0.01 | 0.02 | 0.01 |
|  | BETA | \%MEAN | Calculated vs. Given Disch. (\%) |  |  | Difference (measured vs. pred. WSELs) |  |  |
| SITE | COEFF. | ERROR | 6454 cfs | 9428 cfs | 15049 cfs | 6454 cfs | 9428 cfs | 15049 cfs |
| 81 | 1.97 | 1.1 | 0.9 | 1.7 | 0.8 | 0.02 | 0.06 | 0.03 |
|  | BETA | \%MEAN | Calculated vs. Given Disch. (\%) |  |  | Difference (measured vs. pred. WSELs) |  |  |
| SLTE | COEFF. | ERROR | 6244 cfs | 9035 cfs | 14580 cfs | 6244 cfs | 9035 cfs | 14580 cfs |
| 85 | 3.16 | 2.7 | 2.6 | 4.2 | 1.5 | 0.04 | 0.07 | 0.03 |
|  | BETA | \%MEAN | Calculated vs. Given Disch. (\%) |  |  | Difference (measured vs. pred. WSELs) |  |  |
| SITE | COEFF. | ERROR | 6244 cfs | $\underline{9035 \mathrm{cfs}}$ | 14988 cfs | 6244 cfs | $\underline{9035 \mathrm{cfs}}$ | 14988 cfs |
| 86 | 3.50 | 2.2 | 2.1 | 3.3 | 1.2 | 0.06 | 0.10 | 0.04 |


| SITE | $\begin{gathered} \text { BETA } \\ \text { COEFF. } \end{gathered}$ | \%MEAN <br> ERROR | Calculate 5977 cfs | v. Given Disch. (\%) 10109 cfs | Difference (measured vs. pred. WSELs) |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 87 | 9.22 | --- | --- | --- | 0.00 | 0.00 |  |
| 90 | 3.26 | --- | --- | --- | 0.00 | 0.00 |  |
|  | BETA | \%MEAN | Calculate | s. Given Disch. (\%) | Difference (measured vs. pred. WSELs) |  |  |
| SITE | COEFF. | ERROR | 6106 cfs | 10109 cfs 14988 cfs | 6106 cfs | 10109 cfs | 14988 cfs |
| 88 | 4.83 | 4.1 | 1.8 | $6.4 \quad 4.3$ | 0.01 | 0.05 | 0.04 |
| 89 | 2.36 | 4.6 | 2.0 | 7.24 .8 | 0.01 | 0.10 | 0.10 |
| SITE | $\begin{aligned} & \text { BETA } \\ & \text { COEFF. } \end{aligned}$ | \%MEAN <br> ERROR | Calculate 10109 cfs | s. Given Disch. (\%) 14988 cfs | Difference (measured vs. pred. WSELs) |  |  |
| 88A | 1.77 | --- | --- | --- | 0.00 | 0.00 |  |
|  | BETA | \%MEAN | Calculate | s. Given Disch. (\%) | Difference (measured vs. pred. WSELs) |  |  |
| SITE | COEFF. | ERROR | 6511 cfs | 9606 cfs 15071 cfs | 6511 cfs | $\underline{9606 \mathrm{cfs}}$ | 15071 cfs |
| 94 | 2.72 | 0.6 | 0.5 | $1.0 \quad 0.4$ | 0.02 | 0.03 | 0.02 |
|  | BETA | \%MEAN | Calculate | s. Given Disch. (\%) | Difference (measured vs. pred. WSELs) |  |  |
| SITE | COEFF. | ERROR | 6106 cfs | 9239 cfs 14988 cfs | 6106 cfs | 9239 cfs | 14988 cfs |
| 97 | 4.84 | 3.1 | 2.8 | $4.8 \quad 1.9$ | 0.04 | 0.07 | 0.03 |

## APPENDIX C BRIDGE RIFFLE VELOCITY VALIDATION STATISTICS

Measured Velocities less than $3 \mathrm{ft} / \mathrm{s}$
Difference (measured vs. pred. velocities, $\mathrm{ft} / \mathrm{s}$ )

| Number of Observations | Average | Standard Deviation | Maximum |
| :---: | :---: | :---: | :---: |
| 87 | 1.72 | 1.67 | 6.15 |

Measured Velocities greater than $3 \mathrm{ft} / \mathrm{s}$

Percent Difference (measured vs. pred. velocities)

| Number of Observations | Average | Standard Deviation | Maximum |
| :---: | :---: | :---: | :---: |
| 304 | $30 \%$ | $25 \%$ | $107 \%$ |

[^16] USFWS, SFWO, Energy Planning and Instream Flow Branch





Bridge Riffle Site


## APPENDIXD JUVENILE STRANDING RESULTS

若 哭

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\begin{aligned}
& \stackrel{\rightharpoonup}{\mathrm{N}} \\
& \stackrel{N}{\mathrm{~N}}
\end{aligned}
$$

$$
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$$


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Sacramento River（Keswick Dam to Battle Creek）Redd Dewatering and Juvenile Stranding Final Report
Jurie 22， 2006

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| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
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| GzEO1 | 8SLOL | ttror | SELO1 | 86001 | 6866 | $0<96$ | 8988 | 6Z62 | 6982 | 0¢Zt |
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| 9829 | 8199 | †099 | 9699 | $8 \mathrm{GG9}$ | 6ヶt9 | 0¢19 | 6IES | $688 t$ | $618 \downarrow$ | OGLt |
| 0299 | £St9 | $6 ¢ \succ 9$ | $0 \varepsilon ャ 9$ | £6£9 | 七829 | †969 | ESLG | ってで | 七Gレヤ | 0009 |
| 0099 | £๕๕9 | 6189 | 01E9 | عLZ9 | ャ919 | G789 | $\varepsilon \in 0 \mathrm{G}$ | カ0レヤ | －$\downarrow$ ¢0ャ | 09ZS |
| St69 | 8LLG | ¢929 | gcls | 81LG | 6099 | 0639 | $6 \angle t\rangle$ | 6 6 ¢ $¢$ | 6LもE | 009s |
| 6818 | 乙て0¢ | $800 \varepsilon$ | 6662 | Z962 | £G8Z | カ®GZ | とてLし | ع6L | £ L | 0009 |
| $629 Z$ | とトゥZ | $66 \varepsilon 乙$ | 06๕乙 | £G\＆ะ | ¢ヵてZ | 七Z61 | とトレ | ع8। | カレ | 0099 |
| S8tて | 81とZ | ャ0¢Z | 96ZZ | 8GZZ | 6trz | 0¢81 | 8101 | 68 |  | －0002 |
| ¢トセて | $6 \dagger$ ¢ | †モて乙 | 9てZ乙 | 88レて | 6LOZ | 09 11 | 676 |  |  | 00GL |
| 99ヶレ | 00\＆1 | 98て1 | LLZL | 0ちてし | 1ع1！ | 1.18 |  |  |  | 0008 |
| ¢c9 | 68 t | $\square \angle \downarrow$ | 99t | 8 8t | －618 |  |  |  |  | 0006 |
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Rearing flow (cfs)

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Stranding flow（cfs）

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| L8ZL | OZLL | 901L | 1012 | 0902 | lG69 | 1899 | 0179 | 18Z9 | レヵてG | 09 27 |
| LUG9 | 0G\＆9 | $98 ¢ 9$ | เع६9 | 0629 | 1819 | 1989 | $0 \bullet \square G$ | OLSt | いくカカ | 0009 |
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| szoz | 6981 | 9781 | 0781 | 664. | 0691 | 0LEL | 676 |  |  | 00GL |
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## APPENDIXE

REDD DEWATERING RESULTS

USFWS, SFWO, Energy Planning and Instream Flow Branch
Sacramento River (Keswick Dam to Battle Creek) Redd Dewatering and Juvenile Stranding Final Report















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USFWS, SFWO, Energy Plaming and Instream Flow Branch
Sacramento River (Keswick Dam to Battle Creek) Redd Dewatering and Juvenile Stranding Final Report
June 22, 2006
Percentage of Fall－run Chinook Salmon Redds Dewatered－ACID Dam Boards Out（continued）
$\begin{array}{cccccccccccc}12000 & 13000 & 14000 & 15000 & 17000 & 19000 & 21000 & 23000 & 25000 & 27000 & 29000 & 31000\end{array}$
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 Spawning Flow（cfs）




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Sacramento River (Keswick Dam to Battle Creek) Redd Dewatering and Juvenile Stranding Final Report
June 22, 2006 USFWS, SFWO, Energy Planning and Instream Fiow Branch









Percentage of Late-Fall-run Chinook Salmon Redds Dewatered - ACID Dam Boards Out (continued)




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Percentage of Late-Fall-run Chinook Salmon Redds Dewatered - ACID Dam Boards In (continued)



 $\begin{array}{llllllll}12000 & 13000 & 14000 & 15000 & 17000 & 19000 & 21000 & 23000\end{array}$ Spawning Flow (cfs)









 USFWS, SFWO, Energy Planning and Instream Flow Branch
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Dewatering flow (cfs)

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Spawning Flow (cfs)
$\begin{array}{llllllll}3750 & 4000 & 4250 & 4500 & 4750 & 5000 & 5250 & 5500\end{array}$







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Percentage of Winter－run Chinook Salmon Redds Dewatered－ACID Dam Boards Out（continued） $\begin{array}{ccc}27000 & 29000 & 31000 \\ & & 0.6 \% \\ & 1.6 \% & 3.6 \% \\ -0.6 \% & 8.3 \% & 15.2 \%\end{array}$


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Dewatering flow (cfs)

Percentage of Winter-run Chinook Salmon Redds Dewatered - ACID Dam Boards In (continued) 31000
$0.3 \%$
Sacramento River (Keswick Dam to Battle Creek) Redd Dewatering and Juvenile.Stranding Final Report
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Sacramento River (Keswick Dam to Battle Creek) Redd Dewatering and Juvenile Stranding Final Report



## Dewatering flow (cfs)



## APPENDIX F <br> REVISED SITE SPAWNING HABITAT MODELING RESULTS

Salt Creek Study Site Boards Out WUA ( $\mathrm{ft}^{2}$ )

| Flow (cfs) | Fall-run | Late-fall-run | Winter-run | Steelhead |
| :---: | :---: | :---: | :---: | :---: |
| 3,250 | 4,612 | 2,586 | 1,411 | 1,228 |
| 3,500 | 4,481 | 2,439 | 1,521 | 1,216 |
| 3,750 | 4,354 | 2,303 | 1,581 | 1,214 |
| 4,000 | 4,196 | 2,160 | 1,612 | 1,200 |
| 4,250 | 4,027 | 2,031 | 1,628 | 1,191 |
| 4,500 | 3,854 | 1,919 | 1,627 | 1,193 |
| 4,750 | 3,699 | 1,832 | 1,613 | 1,196 |
| 5,000 | 3,564 | 1,764 | 1,595 | 1,199 |
| 5,250 | 3,426 | 1,701 | 1,567 | 1,192 |
| 5,500 | 3,286 | 1,642 | 1,524 | 1,185 |
| 6,000 | 3,376 | 1,049 | 353 | 1,222 |
| 6,500 | 2,750 | 1,428 | 1,363 | 1,143 |
| 7,000 | 2,472 | 1,291 | 1,230 | 1,101 |
| 7,500 | 2,251 | 1,191 | 1,111 | 1,007 |
| 8,000 | 2,082 | 1,134 | 1,010 | 910 |
| 9,000 | 1,738 | 996 | 863 | 660 |
| 10,000 | 1,464 | 868 | 731 | 573 |
| 11,000 | 1,255 | 767 | 636 | 532 |
| 12,000 | 1,099 | 682 | 557 | 486 |
| 13,000 | 1,006 | 637 | 511 | 461 |
| 14,000 | 960 | 623 | 483 | 451 |
| 15,000 | 868 | 594 | 462 | 434 |
| 17,000 | 705 | 522 | 441 | 383 |
| 19,000 | 809 | 640 | 537 | 383 |
| 21,000 | 693 | 588 | 496 | 330 |
| 23,000 | 891 | 802 | 616 | 404 |
| 25,000 | 780 | 733 | 698 | 400 |
| 27,000 | 978 | 937 | 963 | 398 |
| 29,000 | 913 | 878 | 854 | 360 |
| 31,000 | 858 | 830 | 857 | 321 |

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Salt Creek Study Site Boards In WUA (ft ${ }^{2}$ )

| Flow (cfs) | Fall-run | Late-fall-run | Winter-run | Steelhead |
| :---: | :---: | :---: | :---: | :---: |
| 3,250 | 4,601 | 2,489 | 1,239 | 1,282 |
| 3,500 | 4,519 | 2,367 | 1,338 | 1,240 |
| 3,750 | 4,412 | 2,235 | 1,408 | 1,234 |
| 4,000 | 4,397 | 2,178 | 1,407 | 1,209 |
| 4,250 | 4,219 | 2,035 | 1,454 | 1,227. |
| 4,500 | 4,110 | 1,955 | 1,459 | 1,241 |
| 4,750 | 3,976 | 1,876 | 1,463 | 1,238 |
| 5,000 | 3,819 | 1,795 | 1,449 | 1,227 |
| 5,250 | 3,648 | 1,709 | 1,417 | 1,221 |
| 5,500 | 3,478 | 1,623 | 1,371 | 1,207 |
| 6,000 | 3,640 | 1,270 | 509 | 1,210 |
| 6,500 | 3,433 | 1,099 | 418 | 1,195 |
| 7,000 | 3,226 | 960 | 330 | 1,190 |
| 7,500 | 2,939 | 767 | 250 | 1,162 |
| 8,000 | 2,214 | 1,115 | 955 | 1,040 |
| 9,000 | 1,832 | 936 | 771 | 789 |
| 10,000 | 1,547 | 816 | 640 | 603 |
| 11,000 | 1,348 | 734 | 545 | 573 |
| 12,000 | 1,225 | 695 | 492 | 558 |
| 13,000 | 1,142 | 704 | 499 | 546 |
| 14,000 | 1,007 | 643 | 453 | 490 |
| 15,000 | 1,422 | 848 | 522 | 801 |
| 17,000 | 1,156 | 808 | 569 | 598 |
| 19,000 | 1,048 | 823 | 615 | 520 |
| 21,000 | 981 | 834 | 622 | 486 |
| 23,000 | 1,134 | 1,030 | 936 | 502 |
| 25,000 | 1,046 | 977 | 879 | 446 |
| 27,000 | 1,083 | 1,028 | 1,048 | 534 |
| 29,000 | 892 | 853 | 920 | 377 |
| 31,000 | 873 | 842 | 893 | 444 |

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Upper Lake Redding Study Site Boards Out WUA ( $\mathrm{ft}^{2}$ )

| Flow (cfs) | Fall-run | Late-fall-run | Winter-run | Steelhead |
| :---: | :---: | :---: | :---: | :---: |
| 3,250 | 205,573 | 187,719 | 144,932 | 48,196 |
| 3,500 | 199,927 | 182,105 | 152,226 | 46,763 |
| 3,750 | 193,214 | 175,813 | 158,530 | 45,653 |
| 4,000 | 185,327 | 168,820 | 164,003 | 44,769 |
| 4,250 | 177,601 | 162,053 | 168,119 | 44,112 |
| 4,500 | 169,639 | 155,168 | 171,384. | 43,606 |
| 4,750 | 161,428 | 148,229 | 173,808 | 43,207 |
| 5,000 | 150,847 | 139,932 | 176,103 | 42,938 |
| 5,250 | 143,725 | 133,845 | 176,545 | 42,862 |
| 5,500 | 135,913 | 127,606 | 176,685 | 42,851 |
| 6,000 | 119,816 | 115,290 | 175,220 | 42,841 |
| 6,500 | 105,647 | 104,548 | 171,934 | 42,754 |
| 7,000 | 92,469 | 94,883 | 167,505 | 42,399 |
| 7,500 | 80,488 | 86,220 | 162,376 | 41,731 |
| 8,000 | 70,069 | 78,667 | 156,030 | 40,707 |
| 9,000 | 53,400 | 66,211 | 142,066 | 37,820 |
| 10,000 | 41,117 | 56,837 | 127,929 | 33,768 |
| 11,000 | 32,055 | 49,284 | 113,868 | 28,661 |
| 12,000 | 25,698 | 43,433 | 100,691 | 23,241 |
| 13,000 | 20,979 | 38,886 | 88,343 | 17,315 |
| 14,000 | 17,541 | 35,126 | 77,460 | 11,723 |
| 15,000 | 14,686 | 31,743 | 67,418 | 7,114 |
| 17,000 | 11,292 | 26,571 | 50,825 | 1,444 |
| 19,000 | 9,734 | 22,390 | 38,509 | 292 |
| 21,000 | 8,861 | 18,619 | 29,738 | 183 |
| 23,000 | 7,706 | 14,822 | 22,993 | 140 |
| 25,000 | 6,662 | 11,217 | 18,263 | 128 |
| 27,000 | 5,491 ${ }^{\text {. }}$ | 8,265 | 14,643 | 111 |
| 29,000 | 3,867 | 5,226 | 11,637 | 82 |
| 31,000 | 2,014 | 2,870 | 9,404 | 65 |

USFWS, SFWO, Energy Planning and Instream Flow Branch

Upper Lake Redding Study Site Boards In WUA ( $\mathrm{ft}^{2}$ )

| Flow (cfs) | Fall-run | Late-fall-run | Winter-run | Steelhead |
| :---: | :---: | :---: | :---: | :---: |
| 3,250 | 151,666 | 125,042 | 39,306 | 78,678 |
| 3,500 | 164,283 | 133,532 | 44,252 | 76,016 |
| 3,750 | 175,511 | 140,956 | 49,198 | 73,226 |
| 4,000 | 185,553 | 147,399 | 54,219 | 70,263 |
| 4,250 | 193,612 | 152,431 | 59,035 | 67,310 |
| 4,500 | 200,110 | 156,224 | 63,948 | 64,476 |
| 4,750 | 205,260 | 158,788 | 68,765 | 61,837 |
| 5,000 | 208,977 | 160,318 | 73,301 | 59,369 |
| 5,250 | 211,499 | 160,879 | 77,568 | 57,106 |
| 5,500 | 213,158 | 160,803 | 81,759 | 54,973 |
| 6,000 | 213,524 | 158,325 | 89,258 | 51,385 |
| 6,500 | 210,195 | 153,724 | 96,090 | 48,691 |
| 7,000 | 204,420 | 147,507 | 101,660 | 46,720 |
| 7,500 | 196,791 | 140,417 | 106,315 | 45,136 |
| 8,000 | 188,193 | 132,789 | 110,043 | 43,961 |
| 9,000 | 169,240 | 117,025 | 114,353 | 43,002 |
| 10,000 | 149,770 | 102,490 | 115,064 | 42,938 |
| 11,000 | 131,733 | 89,689 | 112,457 | 42,938 |
| 12,000 | 114,806 | 78,484 | 108,233 | 42,948 |
| 13,000 | 100,012 | 68,991 | 102,641 | 42,959 |
| 14,000 | 86,996 | 60,932 | 96,197 | 42,970 |
| 15,000 | 76,361 | 54,133 | 89,528 | 42,981 |
| 17,000 | 59,445 | 42,884 | 75,219 | 42,895 |
| 19,000 | 46,838 | 34,393 | 61,287 | 42,259 |
| 21,000 | 38,035 | 27,411 | 48,131 | 41,030 |
| 23,000 | 32,023 | 21,690 | 35,999 | 39,231 |
| 25,000 | 27,788 | 16,992 | 25,127 | 36,667 |
| 27,000 | 23,995 | 12,951 | 15,602 | 33,068 |
| 29,000 | 21,377 | 9,331 | 7,783 | 29,620 |
| 31,000 | 19,308 | 6,164 | 2,929 | 26,646 |

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Lower Lake Redding Study Site Boards Out WUA (ft ${ }^{2}$ )

| Flow (cfs) | Fall-run | Late-fall-run | Winter-run | Steelhead |
| :---: | :---: | :---: | :---: | :---: |
| 3,250 | 19,287 | 30,008 | 22,972 | 5,732 |
| 3,500 | 18,910 | 29,598 | 23,037 | 5,657 |
| 3,750 | 18,640 | 29,167 | 23,069 | 5,463 |
| 4,000 | 18,490 | 28,726 | 23,047 | 5,269 |
| 4,250 | 18,403 | 28,273 | 23,144 | 4,967 |
| 4,500 | 18,188 | 27,627 | 23,144 | 4,698 |
| 4,750 | 17,940 | 26,905 | 23,241 | 4,579 |
| 5,000 | 17,660 | 26,064 | 23,392 | 4,472 |
| 5,250 | 17,347 | 25,288 | 23,608 | 4,310 |
| 5,500 | 17,164 | 24,696 | 23,931 | 4,202 |
| 6,000 | 16,561 | 23,327 | 24,340 | 4,051 |
| 6,500 | 15,149 | 21,377 | 23,952 | 3,771 |
| 7,000 | 13,565 | 19,384 | 23,306 | 3,491 |
| 7,500 | 12,111 | 17,574 | 22,325 | 3,362 |
| 8,000 | 11,033 | 16,076 | 21,313 | 3,308 |
| 9,000 | 9,719 | 13,921 | 19,265 | 3,125 |
| 10,000 | 8,835 | 12,510 | 17,337 | 3,049 |
| 11,000 | 8,781 | 12,003 | 15,699 | 3,028 |
| 12,000 | 8,200 | 11,109 | 14,460 | 2,855 |
| 13,000 | 8,275 | 10,850 | 13,275 | 2,888 |
| 14,000 | 8,361 | 10,656 | 12,316 | 2,823 |
| 15,000 | 8,361 | 10,408 | 11,583 | 2,715 |
| 17,000 | 9,072 | 10,408 | 10,753 | 2,942 |
| 19,000 | 10,861 | 11,982 | 10,658 | 3,834 |
| 21,000 | 12,553 | 13,900 | 10,947 | 4,881 |
| 23,000 | 14,955 | 16,776 | 12,391 | 6,196 |
| 25,000 | 26,969 | 29,803 | 16,098 | 11,454 |
| 27,000 | 41,451 | 45,879 | 25,159 | 12,833 |
| 29,000 | 48,519 | 54,176 | 37,981 | 12,176 |
| 31,000 | 45,017 | 50,846 | 48,745 | 11,346 |

Lower Lake Redding Study Site Boards In WUA ( $\mathrm{ft}^{2}$ )

| Flow (cfs) | Fall-run | Late-fall-run | Winter-run | Steelhead |
| :---: | :---: | :---: | :---: | :---: |
| 3,250 | 31,549 | 29,232 | 6,820 | 16,464 |
| 3,500 | 34,199 | 31,441 | 7,876 | 16,593 |
| 3,750 | 36,516 | 33,359 | 8,978 | 16,572 |
| 4,000 | 38,595 | 35,040 | 10,161 | 16,593 |
| 4,250 | 40,481 | 36,527 | 11,303 | 17,013 |
| 4,500 | 42,270 | 37,970 | 12,391 | 17,229 |
| 4,750 | 43,929 | 39,296 | 13,372 | 17,962 |
| 5,000 | 45,782 | 40,912 | 14,330 | 19,265 |
| 5,250 | 47,592 | 42,528 | 15,203 | 20,224 |
| 5,500 | 49,068 | 43,821 | 16,044 | 20,709 |
| 6,000 | 51,385 | 45,847 | 17,595 | 21,517 |
| 6,500 | 53,012 | 47,215 | 18,942 | 22,034 |
| 7,000 | 54,187 | 48,174 | 20,063 | 22,595 |
| 7,500 | 55,145 | 48,971 | 21,032 | 23,177 |
| 8,000 | 56,104 | 49,747 | 21,765 | 23,769 |
| 9,000 | 56,449 | 50,060 | 22,703 | 24,728 |
| 10,000 | 58,787 | 52,085 | 23,963 | 24,373 |
| 11,000 | 62,386 | 55,372 | 26,635 | 25,547 |
| 12,000 | 64,994 | 57,193 | 29,297 | 25,245 |
| 13,000 | 67,569 | 58,744 | 31,064 | 24,879 |
| 14,000 | 69,788 | 59,876 | 32,400 | 24,254 |
| 15,000 | 71,523 | 60,522 | 33,564 | 23,511 |
| 17,000 | 73,215 | 59,994 | 35,320 | 21,884 |
| 19,000 | 72,579 | 57,311 | 36,182 | 20,429 |
| 21,000 | 69,972 | 53,077 | 36,246 | 19,071 |
| 23,000 | 66,028 | 47,980 | 35,600 | 17,929 |
| 25,000 | 61,373 | 42,539 | 34,415 | 17,046 |
| 27,000 | 56,492 | 37,281 | 32,626 | 16,485 |
| 29,000 | 51,730 | 32,454 | 30,288 | 16,119 |
| 31,000 | 47,161 | 28,165 | 27,821 | 15,828 |

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Bridge Riffle Study Site WUA ( $\mathrm{ft}^{2}$ )

| Flow (cfs) | Fall-run | Late-fall-run | Winter-run | Steelhead |
| :---: | :---: | :---: | :---: | :---: |
| 3,250 | 3,416 | 4,525 | 3,200 | 1,562 |
| 3,500 | 3,453 | 4,649 | 3,267 | 1,684 |
| 3,750 | 3,069 | 4,312 | 2,971 | 1,647 |
| 4,000 | 3,088 | 4,353 | 2,807 | 1,766 |
| 4,250 | 3,307 | 4,589 | 2,851 | 1,766 |
| 4,500 | 3,794 | 5,081 | 3,022 | 1,961 |
| 4,750 | 4,291 | 5,539 | 3,295 | 2,028 |
| 5,000 | 4,839 | 6,079 | 3,314 | 2,155 |
| 5,250 | 5,182 | 6,378 | 3,215 | 2,402 |
| 5,500 | 5,145 | 6,352 | 3,313 | 2,370 |
| 6,000 | 5,882 | 6,957 | 3,575 | 2,578 |
| 6,500 | 6,499 | 7,514 | 4,167 | 2,653 |
| 7,000 | 6,667 | 7,646 | 4,495 | 2,697 |
| 7,500 | 6,791 | 7,698 | 4,736 | 2,725 |
| 8,000 | 7,153 | 7,973 | 5,320 | 2,660 |
| 9,000 | 6,624 | 7,406 | 6,067 | 2,506 |
| 10,000 | 4,909 | 6,519 | 10,446 | 2,116 |
| 11,000 | 3,987 | 5,691 | 9,652 | 1,823 |
| 12,000 | 3,506 | 5,193 | 8,512 | 1,534 |
| 13,000 | 3,109 | 4,722 | 7,385 | 1,214 |
| 14,000 | 2,971 | 4,392 | 6,382 | 1,018 |
| 15,000 | 2,889 | 4,101 | 5,475 | 985 |
| 17,000 | 2,695 | 3,617 | 4,797 | 881 |
| 19,000 | 2,555 | 3,095 | 4,167 | 908 |
| 21,000 | 2,230 | 2,644 | 3,436 | 905 |
| 23,000 | 2,475 | 2,659 | 3,255 | 901 |
| 25,000 | 2,047 | 2,178 | 2,197 | 787 |
| 27,000 | 2,100 | 2,106 | 2,400 | 622 |
| 29,000 | 448 | 868 | 1,687 | 112 |
| 31,000 | 318 | 608 | 1,477 | 125 |

Posse Grounds Study Site WUA (ft²)

| Flow (cfs) | Fall-run | Late-fall-run | Winter-run | Steelhead |
| :---: | :---: | :---: | :---: | :---: |
| 3,250 | 32,594 | 58,647 | 30,730 | 8,318 |
| 3,500 | 35,428 | 64,035 | 34,016 | 8,501 |
| 3,750 | 37,949 | 68,474 | 37,011 | 8,491 |
| 4,000 | 40,050 | 72,094 | 39,824 | 8,447 |
| 4,250 | 41,688 | 74,766 | 42,367 | 8,329 |
| 4,500 | 42,701 | 76,232 | 44,791 | 8,286 |
| 4,750 | 43,315 | 77,137 | 47,269 | 8,254 |
| 5,000 | 43,272 | 76,867 | 49,489 | 8,243 |
| 5,250 | 43,207 | 76,350 | 51,859 | 8,297 |
| 5,500 | 42,560 | 74,960 | 53,993 | 8,534 |
| 6,000 | 40,599 | 71,060 | 57,839 | 8,814 |
| 6,500 | 38,046 | 66,621 | 60,878 | 9,019 |
| 7,000 | 35,525 | 62,742 | 62,968 | 9,245 |
| 7,500 | 33,025 | 59,455 | 64,563 | 9,331 |
| 8,000 | 30,557 | 56,341 | 65,435 | 9,266 |
| 9,000 | 26,506 | 51,234 | 65,349 | 9,266 |
| 10,000 | 22,950 | 46,019 | 62,860 | 8,394 |
| 11,000 | 20,052 | 41,612 | 59,197 | 7,305 |
| 12,000 | 17,595 | 37,496 | 54,564 | 6,053 |
| 13,000 | 14,912 | 33,854 | 49,607 | 4,946 |
| 14,000 | 13,005 | 31,010 | 45,211 | 4,148 |
| 15,000 | 11,367 | 28,413 | 41,278 | 3,459 |
| 17,000 | 8,932 | 23,855 | 34,307 | 2,554 |
| 19,000 | 6,950 | 19,610 | 27,724 | 1,681 |
| 21,000 | 5,937 | 16,227 | 22,713 | 1,153 |
| 23,000 | 4,816 | 12,876 | 18,597 | 813 |
| 25,000 | 3,933 | 9,676 | 15,300 | 638 |
| 27,000 | 2,985 | 6,691 | 12,628 | 584 |
| 29,000 | 1,659 | 3,556 | 10,215 | 398 |
| 31,000 | 1,065 | 2,047 | 8,340 | 255 |

Above Hawes Hole Study Site WUA ( $\mathrm{ft}^{2}$ )

| Flow (cfs) | Fall-run | Late-fall-run | Winter-run | Steelhead |
| :---: | :---: | :---: | :---: | :---: |
| 3,250 | 75,294 | 91,651 | 84,905 | 22,940 |
| 3,500 | 77,277 | 93,277 | 87,168 | 23,866 |
| 3,750 | 78,839 | 94,420 | 89,355 | 24,340 |
| 4,000 | 79,669 | 94,894 | 90,993 | 24,200 |
| 4,250 | 80,693 | 95,616 | 92,879 | 24,157 |
| 4,500 | 81,371 | 95,939 | 94,549 | 24,243 |
| 4,750 | 81,393 | 95,659 | 95,723 | 24,179 |
| 5,000 | 81,317 | 95,379 | 96,575 | 24,039 |
| 5,250 | 80,682 | 94,624 | 96,725 | 23,985 |
| 5,500 | 80,617 | 94,312 | 97,221 | 24,254 |
| 6,000 | 79,604 | 93,288 | 97,189 | 24,082 |
| 6,500 | 78,354 | 91,521 | 96,736 | 23,726 |
| 7,000 | 75,725 | 88,817 | 95,971 | 22,950 |
| 7,500 | 71,868 | 85,078 | 94,926 | 22,131 |
| 8,000 | 67,192 | 80,757 | 93,935 | 21,463 |
| 9,000 | 58,270 | 72,848 | 94,948 | 20,192 |
| 10,000 | 48,326 | 63,875 | 94,646 | 18,885 |
| 11,000 | 39,331 | 55,242 | 89,952 | 17,495 |
| 12,000 | 32,114 | 47,693 | 83,867 | 15,717 |
| 13,000 | 26,349 | 41,387 | 77,192 | 13,910 |
| 14,000 | 21,311 | 35,706 | 70,592 | 11,968 |
| 15,000 | 17,675 | 31,101 | 62,942 | 9,841 |
| 17,000 | 13,845 | 26,125 | 52,391 | 7,246 |
| 19,000 | 12,061 | 23,028 | 44,034 | 5,367 |
| 21,000 | 11,192 | 20,900 | 37,879 | 4,216 |
| 23,000 | 9,707 | 18,135 | 33,473 | 3,065 |
| 25,000 | 7,841 | 15,001 | 29,257 | 2,247 |
| 27,000 | 6,787 | 12,846 | 25,495 | 1,952 |
| 29,000 | 5,996 | 11,086 | 22,330 | 1,730 |
| 31,000 | 5,171 | 9,494 | 19,463 | 1,474 |

Powerline Riffle Study Site WUA ( $\mathrm{ft}^{2}$ )

| Flow (cfs) | Fall-run | Late-fall-run | Winter-run | Steelhead |
| :---: | :---: | :---: | :---: | :---: |
| 3,250 | 47,420 | 60,414 | 74,206 | 10,764 |
| 3,500 | 45,427 | 58,540 | 75,402 | 10,727 |
| 3,750 | 43,649 | 56,826 | 76,566 | 10,650 |
| 4,000 | 41,354 | 54,617 | 77,191 | 10,443 |
| 4,250 | 39,177 | 52,495 | 77,557 | 10,287 |
| 4,500 | 36,591 | 49,995 | 77,439 | 10,057 |
| 4,750 | 34,135 | 47,625 | 77,094 | 9,833 |
| 5,000 | 31,624 | 45,179 | 76,361 | 9,592 |
| 5,250 | 29,146 | 42,711 | 75,337 | 9,357 |
| 5,500 | 26,862 | 40,449 | 74,282 | 9,119 |
| 6,000 | 22,530 | 36,096 | 71,308 | 8,534 |
| 6,500 | 18,974 | 32,454 | 67,720 | 7,827 |
| 7,000 | 16,033 | 29,307 | 63,787 | 6,950 |
| 7,500 | 13,587 | 26,625 | 59,649 | 6,033 |
| 8,000 | 11,610 | 24,308 | 55,286 | 5,103 |
| 9,000 | 8,791 | 20,698 | 47,097 | 3,404 |
| 10,000 | 7,010 | 18,048 | 39,748 | 2,110 |
| 11,000 | 6,004 | 16,184 | 33,833 | 1,414 |
| 12,000 | 5,330 | 14,621 | 29,200 | 1,025 |
| 13,000 | 4,869 | 13,275 | 25,332 | 782 |
| 14,000 | 4,395 | 11,949 | 22,164 | 533 |
| 15,000 | 3,958 | 10,623 | 19,276 | 339 |
| 17,000 | 3,551 | 8,458 | 15,031 | 238 |
| 19,000 | 3,149 | 6,616 | 12,068 | 217 |
| 21,000 | 2,745 | 5,007 | 9,726 | 227 |
| 23,000 | 1,733 | 3,159 | 7,904 | 196 |
| 25,000 | 967 | 1,751 | 6,614 | 143 |
| 27,000 | 637 | 1,088 | 5,408 | 71 |
| 29,000 | 376 | 713 | 4,392 | 25 |
| 31,000 | 266 | 463 | 3,537 | 0.11 |

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Price Riffle Study Site WUA (ft²)

| Flow (cfs) | Fall-run | Late-fall-run | Winter-run | Steelhead |
| :---: | :---: | :---: | :---: | :---: |
| 3,250 | 38,078 | 63,162 | 48,110 | 5,183 |
| 3,500 | 40,298 | 66,276 | 52,894 | 6,260 |
| 3,750 | 40,729 | 66,384 | 54,402 | 7,106 |
| 4,000 | 40,686 | 65,974 | 55,727 | 7,730 |
| 4,250 | 40,082 | 65,241 | 57,925 | 8,254 |
| 4,500 | 39,468 | 64,703 | 60,285 | 8,915 |
| 4,750 | 38,552 | 63,787 | 62,731 | 9,603 |
| 5,000 | 38,024 | 63,280 | 65,522 | 10,253 |
| 5,250 | 37,852 | 63,140 | 66,858 | 10,904 |
| 5,500 | 37,119 | 62,612 | 68,183 | 11,335 |
| 6,000 | 35,050 | 60,414 | 69,390 | 11,971 |
| 6,500 | 33,456 | 58,701 | 69,702 | 12,714 |
| 7,000 | 32,669 | 57,257 | 70,500 | 13,188 |
| 7,500 | 32,712 | 56,288 | 69,702 | 13,081 |
| 8,000 | 32,421 | 55,210 | 68,851 | 12,887 |
| 9,000 | 34,318 | 54,779 | 64,972 | 12,790 |
| 10,000 | 35,869 | 55,016 | 65,478 | 13,565 |
| 11,000 | 33,844 | 52,085 | 67,030 | 13,328 |
| 12,000 | 31,452 | 49,133 | 67,558 | 13,996 |
| 13,000 | 27,206 | 43,789 | 66,330 | 14,341 |
| 14,000 | 25,084 | 40,686 | 65,522 | 14,374 |
| 15,000 | 24,416 | 38,391 | 64,390 | 14,320 |
| 17,000 | 20,806 | 33,111 | 58,109 | 11,626 |
| 19,000 | 15,009 | 25,547 | 49,402 | 8,552 |
| 21,000 | 11,626 | 20,677 | 42,323 | 6,930 |
| 23,000 | 8,788 | 15,419 | 33,402 | 4,831 |
| 25,000 | 6,493 | 11,928 | 28,155 | 3,722 |
| 27,000 | 5,050 | 9,203 | 23,597 | 3,130 |
| 29,000 | 4,727 | 7,844 | 20,278 | 2,938 |
| 31,000 | 4,364 | 6,767 | 17,972 | 2,898 |

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## APPENDIX G <br> REVISED SEGMENT SPAWNING HABITAT MODELING RESULTS

Segment 6 Boards Out WUA ( $\mathrm{ft}^{2}$ )

| Flow (cfs) | Fall-run | Late-fall-run | Winter-run | Steelhead |
| :---: | :---: | :---: | :---: | :---: |
| 3,250 | 279,956 | 268,782 | 372,493 | 67,290 |
| 3,500 | 272,448 | 261,253 | 388,925 | 65,436 |
| 3,750 | 263,774 | 252,885 | 402,996 | 63,843 |
| 4,000 | 253,776 | 243,641 | 415,056 | 62,510 |
| 4,250 | 244,038 | 234,676 | 424,360 | 61,329 |
| 4,500 | 233,851 | 225,351 | 431,541 | 60,386 |
| 4,750 | 223,342 | 215,899 | 437,056 | 59,758 |
| 5,000 | 209,927 | 204,667 | 442,398 | 59,303 |
| 5,250 | 200,688 | 196,217 | 443,784 | 59,004 |
| 5,500 | 190,763 | 187,812 | 444,708 | 58,850 |
| 6,000 | 170,499 | 170,393 | 439,809 | 58,699 |
| 6,500 | 150,726 | 155,371 | 433,948 | 58,155 |
| 7,000 | 132,377 | 140,981 | 422,490 | 57,329 |
| 7,500 | 115,717 | 128,082 | 408,786 | 56,242 |
| 8,000 | 101,484 | 116,970 | 392,377 | 54,809 |
| 9,000 | 79,126 | 98,976 | 356,827 | 50,758 |
| 10,000 | 62,728 | 85,662 | 321,193 | 45,616 |
| 11,000 | 51,351 | 75,706 | 286,447 | 39,310 |
| 12,000 | 42,696 | 67,373 | 254,558 | 32,430 |
| 13,000 | 36,917 | 61,455 | 224,684 | 25,210 |
| 14,000 | 32,772 | 56,614 | 198,570 | 18,296 |
| 15,000 | 29,176 | 52,149 | 174,819 | 12,521 |
| 17,000 | 25,704 | 45,751 | 136,442 | 5,818 |
| 19,000 | 26,113 | 42,715 | 109,349 | 5,501 |
| 21,000 | 26,971 | 40,391 | 90,598 | 6,581 |
| 23,000 | 28,733 | 39,528 | 79,200 | 8,223 |
| 25,000 | 41,981 | 50,939 | 77,130 | 14,618 |
| 27,000 | 58,462 | 67,199 | 89,683 | 16,277 |
| 29,000 | 65,025 | 73,542 | 111,038 | 15,394 |
| 31,000 | 58,425 | 66,546 | 129,813 | 14,313 |

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Segment 6 Boards In WUA (ft ${ }^{2}$ )

| Flow (cfs) | Fall-run | Late-fall-run | Winter-run | Steelhead |
| :---: | :---: | :---: | :---: | :---: |
| 3,250 | 229,136 | 191,251 | 104,203 | 117,637. |
| 3,500 | 247,661 | 204,155 | 117,625 | 114,496 |
| 3,750 | 264,056 | 215,391 | 131,085 | 111,059 |
| 4,000 | 278,825 | 225,233 | 144,731 | 107,439 |
| 4,250 | 290,741 | 233,011 | 157,942 | 104,371 |
| 4,500 | 300,718 | 239,302 | 171,156 | 101,194 |
| 4,750 | 308,861 | 243,951 | 183,920 | 98,865 |
| 5,000 | 315,465 | 247,691 | 195,976 | 97,430 |
| 5,250 | 320,542 | 250,242 | 207,214 | 95,832 |
| 5,500 | 324,159 | 251,621 | 218,183 | 93,805 |
| 6,000 | 327,630 | 250,639 | 236,196 | 90,417 |
| 6,500 | 325,301 | 246,486 | 253,990 | 87,742 |
| 7,000 | 319,436 | 239,902 | 268,517 | 86,016 |
| 7,500 | 310,948 | 231,989 | 280,713 | 84,760 |
| 8,000 | 300,743 | 224,054 | 292,079 | 83,899 |
| 9,000 | 277,576 | 204,986 | 303,219 | 83,593 |
| 10,000 | 256,327 | 189,577 | 307,267 | 82,855 |
| 11,000 | 238,470 | 177,870 | 307,201 | 84,251 |
| 12,000 | 220,851 | 166,374 | 303,648 | 83,876 |
| 13,000 | 205,842 | 156,696 | 295,249 | 83,428 |
| 14,000 | 192,505 | 148,170 | 283,910 | 82,611 |
| 15,000 | 182,153 | 140,914 | 271,951 | 82,097 |
| 17,000 | 163,256 | 126,497 | 244,438 | 79,760 |
| 19,000 | 146,967 | 112,883 | 215,785 | 77,114 |
| 21,000 | 132,965 | 99,213 | 186,998 | 73,916 |
| 23,000 | 121,006 | 86,254 | 159,577 | 70,348 |
| 25,000 | 110,053 | 73,820 | 132,926 | 66,074 |
| 27,000 | 99,515 | 62,537 | 108,407 | 61,106 |
| 29,000 | 90,279 | 52,018 | 85,780 | 56,262 |
| 31,000 | 82,157 | 42,909 | 69,615 | 52,360 |

Segment 5 WUA ( $\mathrm{ft}^{2}$ )

| Flow (cfs) | Fall-run | Late-fall-run | Winter-run | Steelhead |
| :---: | :---: | :---: | :---: | :---: |
| 3,250 | 517,564 | 493,885 | 569,220 | 152,613 |
| 3,500 | 540,135 | 516,656 | 596,120 | 158,337 |
| 3,750 | 557,335 | 533,387 | 619,524 | 160,323 |
| 4,000 | 571,053 | 546,578 | 640,059 | 160,020 |
| 4,250 | 584,449 | 558,157 | 661,485 | 159,27? |
| 4,500 | 594,577 | 565,434 | 681,914 | 160,379 |
| 4,750 | 599,845 | 568,889 | 700,715 | 160,244 |
| 5,000 | 601,840 | 568,857 | 715,521 | 160,132 |
| 5,250 | 600,180 | 565,753 | 727,117 | 161,281 |
| 5,500 | 596,697 | 560,241 | 740,184 | 163,485 |
| 6,000 | 586,295 | 546,463 | 759,708 | 164,954 |
| 6,500 | 571,480 | 528,443 | 774,931 | 164,601 |
| 7,000 | 548,314 | 507,864 | 782,849 | 162,248 |
| 7,500 | 519,331 | 485,617 | 786,638 | 158,970 |
| 8,000 | 487,794 | 462,776 | 788,865 | 155,259 |
| 9,000 | 425,010 | 419,447 | 796,884 | 148,633 |
| 10,000 | 354,260 | 371,357 | 804,490 | 136,687 |
| 11,000 | 294,671 | 327,119 | 760,657 | 123,797 |
| 12,000 | 247,450 | 288,319 | 703,857 | 108,364 |
| 13,000 | 206,321 | 255,082 | 642,741 | 93,326 |
| 14,000 | 173,385 | 226,835 | 585,266 | 79,673 |
| 15,000 | 148,479 | 202,932 | 525,439 | 66,425 |
| 17,000 | 118,445 | 170,974 | 438,261 | 49,667 |
| 19,000 | 100,282 | 145,888 | 363,681 | 36,995 |
| 21,000 | 90,019 | 126,869 | 306,694 | 29,174 |
| 23,000 | 79,041 | 107,407 | 265,007 | 22,222 |
| 25,000 | 64,268 | 85,667 | 223,952 | 17,075 |
| 27,000 | 55,205 | 69,041 | 194,105 | 14,685 |
| 29,000 | 37,679 | 49,477 | 163,971 | 10,416 |
| 31,000 | 30,476 | 38,755 | 140,251 | 8,621 |

Segment 4 WUA ( $\mathrm{ft}^{2}$ )

| Flow (cfs) | Fall-run | Late-fall-run | Winter-run | Steelhead |
| :---: | :---: | :---: | :---: | :---: |
| 3,250 | 276,159 | 594,401 | 183,474 | 51,509 |
| 3,500 | 276,892 | 600,365 | 192,444 | 54,868 |
| 3,750 | 272,541 | 592,640 | 196,452 | 57,352 |
| 4,000 | 264,989 | 580,043 | 199,377 | 58,699 |
| 4,250 | 256,007 | 566,310 | 203,223 | 59,887 |
| 4,500 | 245,671 | 551,697 | 206,586 | 61,280 |
| 4,750 | 234,779 | 535,892 | 209,738 | 62,778 |
| 5,000 | 224,963 | 521,688 | 212,825 | 64,099 |
| 5,250 | 216,404 | 509,143 | 213,293 | 65,443 |
| 5,500 | 206,659 | 495,723 | 213,698 | 66,066 |
| 6,000 | 185,983 | 464,213 | 211,047 | 66,231 |
| 6,500. | 169,349 | 438,456 | 206,133 | 66,347 |
| 7,000 | 157,307 | 416,373 | 201,431 | 65,046 |
| 7,500 | 149,546 | 398,812 | 194,027 | 61,738 |
| 8,000 | 142,220 | 382,482 | 186,206 | 58,108 |
| 9,000 | 139,242 | 363,044 | 168,104 | 52,307 |
| 10,000 | 138,499 | 351,438 | 157,839 | 50,630 |
| 11,000 | 128,709 | 328,374 | 151,295 | 47,617 |
| 12,000 | 118,806 | 306,657 | 145,137 | 48,518 |
| 13,000 | 103,602 | 274,478 | 137,493 | 48,847 |
| 14,000 | 95,217 | 253,174 | 131,529 | 48,150 |
| 15,000 | 91,648 | 235,757 | 125,499 | 47,349 |
| 17,000 | 78,673 | 199,947 | 109,710 | 38,321 |
| 19,000 | 58,650 | 154,704 | 92,205 | 28,324 |
| 21,000 | 46,418 | 123,540 | 78,074 | 23,117 |
| 23,000 | 33,983 | 89,360 | 61,959 | 16,237 |
| 25,000 | 24,096 | 65,796 | 52,154 | 12,484 |
| 27,000 | 18,369 | 49,500 | 43,508 | 10,339 |
| 29,000 | 16,483 | 41,159 | 37,005 | 9,570 |
| 31,000 | 14,955 | 34,776 | 32,264 | 9,361 |


[^0]:    ${ }^{1}$ PHABSIM is the collection of one dimensional hydraulic and habitat models which are used to predict the relationship between physical habitat availability and streamflow over a range of river discharges.

[^1]:    ${ }^{3}$ We defined the stranding flow as the flow where the connection between the stranding area and main river channel has a maximum depth of 0.1 feet. We selected 0.1 feet because the minimum depth at which we found juvenile salmon during our juvenile HSI data collection was 0.2 feet. When flows drop to or below the stranding flow, juvenile salmon will be isolated from the main river channel.
    ${ }^{4}$ For the remaining site, we were only able to measure WSELs at two flows (10,181 and $14,986 \mathrm{cfs}$, since this site was located on a side channel which stopped flowing when the total river flow dropped below 9,300 cfs.

[^2]:    ${ }^{5}$ The first four characters of the ADCP traverses designates which CDG file (containing the ADCP settings) was used for the traverses.
    ${ }^{6}$ WT is the water track transmit length.
    7 The Transect program is the software used to receive, record and process data from the ADCP .

[^3]:    USFWS, SFWO, Energy Planning and Instream Flow Branch
    Sacramento River (Keswick Dam to Battle Creek) Redd Dewatering and Juvenile Stranding Final Report June 22, 2006

[^4]:    ${ }^{12}$ These flows are the same as the stranding study site flows for those stranding sites that include all of the Sacramento River flow.

[^5]:    USFWS, SFWO, Energy Planning and Instream Flow Branch
    Sacramento River (Keswick Dam to Battle Creek) Redd Dewatering and Juvenile Stranding Final Report June 22, 2006

[^6]:    ${ }^{15}$ The first three criteria are from U.S. Fish and Wildlife Service 1994, while the fourth criterion is our own.
    ${ }^{16}$ For much of the Sacramento River, the WSEL going across the river will differ by more than 0.2 feet.

[^7]:    ${ }^{17} \mathrm{~A}$ is similar to a SZF, but is not strictly speaking a SZF since the WSEL at the location with the maximum depth was significantly lower than the WSEL at the stranding location.
    ${ }^{18} \mathrm{Q}$ is the total river flow, Site 66 Q is the flow in Stranding Site 66, etc.
    ${ }_{19}$ Since only two flows were used in these regressions, the $\mathrm{R}^{2}$-values, by definition, were one.

[^8]:    ${ }^{22}$ This criteria is based on the assumption that flow in low gradient streams is usually subcritical, where the Froude number is less than one (Peter Steffler, personal communication).

[^9]:    ${ }^{23}$ We have selected this standard because it is a standard used for PHABSIM (U. S. Fish and Wildlife Service 2000).
    ${ }^{24}$ Bridge Riffle transect 3 had both a side channel (SC) and a main channel (MC).
    ${ }^{25}$ All WSEL measurements were made on the left bank (LB).
    USFWS, SFWO, Energy Planning and Instream Flow Branch
    Sacramento River (Keswick Dam to Battle Creek) Redd Dewatering and Juvenile Stranding Final Report June 22, 2006

[^10]:    USFW, SFWO, Energy Planning and Instream Flow Branch
    Sacramento River (Keswick Dam to Battle Creek) Redd Dewatering and Juvenile Stranding Final Report June 22, 2006

[^11]:    ${ }^{29}$ Two feet was selected because the drop in stage associated with a change in flow for the Sacramento River sites is typically less than two feet.

[^12]:    ${ }^{30} 11,227$ fish is the total number of juvenile fish for the stranding sites (sites $36,45 \mathrm{~B}, 51$, $51 \mathrm{~A}, 61 \mathrm{~A}$ and 71 ) that strand between 3,750 and $3,250 \mathrm{cfs}$. In the context of this report, juvenile fish refer to any young-of-the-year salmon, generally in the size range of 35 to 100 mm .

[^13]:    USFWS, SFWO, Energy Planning and Instream Flow Branch
    Sacramento River (Keswick Dam to Battle Creek) Redd Dewatering and Juvenile Stranding Final Report

[^14]:    USFWS, SFWO, Energy Planning and Instream Flow Branch
    Sacramento River (Keswick Dam to Battle Creek) Redd Dewatering and Juvenile Stranding Final Report June 22, 2006

[^15]:    USFWS, SFWO, Energy Planning and Instream Flow Branch
    Sacramento River (Keswick Dam to Battle Creek) Redd Dewatering and Juvenile Stranding Final Report June 22, 2006

[^16]:    All differences were calculated as the absolute value of the difference between the measured and simulated velocity.

