

Chapter 9

Geology and Seismicity

9.1 Summary Comparison of Proposed Project

A summary comparison of an impact related to geology and seismicity is provided in Figure 9-0. This figure provides information on an impact related to ground settlement that is expected to result from the proposed project compared with the approved project. These incremental values, together with consideration of the severity of the underlying impacts as set forth in the Final EIR/EIS, are the basis for making both NEPA and CEQA impact significance findings. The incremental analysis addresses whether the proposed project, compared with the approved project, would lead to any new significant environmental effects or to any substantial increase in the severity of previously identified significant effects. The incremental difference between the original impacts and the newly anticipated impacts is then considered against the backdrop of the original significance determinations for the original underlying impacts as described in the Final EIR/EIS.

Figure 9-0. Comparison of Impacts Related to Geology and Seismicity

Chapter 9 – Geology and Seismicity	Approved Project	Proposed Project (Total)	Proposed Project (Increment)
Impact GEO-3: Loss of Property, Personal Injury, or Death from Ground Settlement during Construction of Water Conveyance Features (number of segments that pose greatest risk of settlement per alternative)	2	2	0
	Less than significant/not adverse	Remains less than significant/not adverse. No change from the approved project	

As depicted in Figure 9-0, the proposed project would not result in new impacts or a substantial increase in the severity of previously identified impacts related to geology and seismicity. This chapter contains the information necessary to make the Final EIR/EIS adequate for the approved project as revised.

9.2 Environmental Setting/Affected Environment

9.2.1 Affected Environment

The Existing Conditions related to geology and seismicity that would be affected by construction and operation of the proposed project are slightly different from those described in Final EIR/EIS Chapter 9, *Geology and Seismicity*, Section 9.1 *Environmental Setting/Affected Environment*. Specifically, some of the proposed project facilities would be closer to an earthquake fault that may be capable of surface deformations. The Final EIR/EIS provides a discussion of geologic substrates, seismicity including ground shaking and surface fault rupture, liquefaction, ground settlement, slope instability, seiche, tsunami, and mudflow hazards found within the Plan Area and region. The proposed project would be located entirely within the previously analyzed project area and,

1 consequently, the Existing Conditions have not changed. Figure 9-2, *Geologic Borehole Locations*, is
2 based on boring logs contained in the 2009 through 2012 DWR geotechnical data reports and shows
3 a cross-section of the stratigraphy of the sediments and peat (expressed as Unified Soil Classification
4 System abbreviations) generally oriented along the proposed project alignment.

5 Among the regulatory design codes and standards for project structures that are presented in Final
6 EIR/EIS Section 9.2.2.6, more recent versions have been issued for five.

- 7 • American Association of State Highway and Transportation Officials Guide Specifications for
8 LRFD [load and resistance factor] Seismic Bridge Design, 1st Edition, 2009. Updated as 2nd
9 edition in 2011.
- 10 • American Railway Engineering and Maintenance-of-Way Association *Manual for Railway*
11 *Engineering*, Volume 2, Chapter 9, *Seismic Design for Railway Structures*, 2008. Updated in 2017.
- 12 • California Building Standards Code, 2010 (Title 24 California Code of Regulations). Revised
13 edition published in 2016.
- 14 • California Department of Transportation *Seismic Design Criteria*, Version 1.6, Nov 2010. Updated
15 with Version 1.7 in 2013 and includes changes regarding liquefaction and lateral spreading
16 considerations, new design provisions for pile foundations in poor and marginal soils, and other
17 substantive design issues.
- 18 • U.S. Army Corps of Engineers (USACE) Engineering and Design—*Earthquake Design and*
19 *Evaluation for Civil Works Projects*, ER 1110-2-1806, 1995. Superseded by 2016 version.

20 Proposed project design and construction techniques would consider and adhere to all relevant
21 changes in the requirements of these codes and standards from the versions in effect at the time that
22 the Final EIR/EIS was prepared.

23 9.3 Environmental Consequences

24 This section describes the potential effects of the modifications to the approved project on geology
25 and seismicity within the study area. The focus of this assessment is on determining the incremental
26 effect from geology and seismicity that is attributable to these modifications. With the exception of
27 focusing on the incremental effects, the methods of analysis and determination of effects is the same
28 as indicated in the Final EIR/EIS.

29 Effects are evaluated for severity and, where appropriate, mitigation measures are identified. Where
30 mitigation measures identified in the Final EIR/EIS remain sufficient, such sufficiency is noted. This
31 section describes potential direct and reasonably foreseeable indirect effects on geology and
32 seismicity that would result from construction of the proposed project. Some impact topics
33 addressed in the Final EIR/EIS are not addressed herein because the change in the footprint of the
34 water conveyance facilities would not result in a changed impact. This chapter does not address
35 impacts at the restoration opportunity areas. Additionally, the impacts resulting from
36 implementation of Environmental Commitments 3, 4, 6–12, 15, and 16, whether they occur under
37 the proposed project or approved project, are fully disclosed in the Final EIR/EIS and would not
38 change if the footprint changes described for the proposed project are constructed.

1 Direct or indirect effects from geology and seismicity in areas upstream of the Delta are not
2 anticipated; thus, the geology and seismicity in these areas are not discussed further in this section.
3 Potential effects on upstream areas are discussed in Final EIR/EIS Chapter 5, *Water Supply*.

4 The methods applied to the analysis of impacts on geology and seismicity are the same as indicated
5 in Section 9.3.1 in the Final EIR/EIS.

6 **9.3.1 Effects and Mitigation Approaches**

7 **9.3.1.1 No Action Alternative**

8 Under the No Action Alternative, the new Byron Tract Forebay, reusable tunnel material (RTM)
9 storage, and other footprint changes described for the proposed project would not occur. For the
10 purposes of this Supplemental EIR/EIS, the No Action Alternative, against which this proposed
11 project is compared, is consistent with the No Action Alternative Early Long-Term in the Final
12 EIR/EIS. No differing effects resulting from geologic conditions and seismic hazards would occur
13 along the proposed project alignment from what was previously described in the No Action
14 Alternative Early Long-Term in the Final EIR/EIS if the No Action Alternative were to occur.

15 **9.3.1.2 Proposed Project**

16 The proposed project would result in permanent effects on geologic conditions and the water
17 conveyance facility's susceptibility of seismic hazards in the study area associated with construction
18 of a forebay, tunnels, and canal. Nearby areas would be altered as work or staging areas, concrete
19 batch plants, fuel stations, or be used for spoils storage areas. Transmission lines, access roads, and
20 other incidental facilities would also be needed for operation of the project and construction of these
21 structures would have a permanent effect on the geologic substrate.

22 Implementation of the proposed project would also result in permanent changes in the geologic
23 substrate associated with the water conveyance structures. Other feature modifications that would
24 result in effects on the geologic substrate include soil borrow, spoil, reusable tunnel material storage
25 areas, and access roads.

26 **Impact GEO-1: Loss of Property, Personal Injury, or Death from Structural Failure Resulting** 27 **from Strong Seismic Shaking of Water Conveyance Features during Construction**

28 ***RTM Storage***

29 Changes related to moving RTM storage from Zacharias Island to Bouldin Island under the proposed
30 project would result in a similar impact with respect to potential loss of property, personal injury, or
31 death from structural failure resulting from strong seismic shaking of the RTM storage area as
32 described for the approved project in Final EIR/EIS Section 9.3.4.2, *Alternative 4A*. The RTM itself
33 would be identical in composition and be placed in a manner similar to that for the approved
34 project. The RTM storage site used for the proposed project would have a similar ground shaking
35 potential as the site that would be used for the approved project. Therefore, its potential for failure
36 and consequent potential loss of property, personal injury, or death from caused by seismic shaking
37 at the RTM storage area during construction would be similar to that of the approved project.

1 **Byron Tract Forebay and Conveyance**

2 Earthquakes could be generated from local and regional seismic sources during construction of the
3 proposed project water conveyance facilities. Seismically induced ground shaking could cause injury
4 of workers at the construction sites as a result of collapse of facilities.

5 Changes related to constructing the new Byron Tract Forebay, south tunnels, and canal instead of
6 the Clifton Court Forebay modifications could result in a greater risk to property, personal injury, or
7 death from structural failure resulting from seismic shaking of conveyance features during
8 construction because the Byron Tract Forebay would be located closer to a potential shaking source
9 (i.e., the West Tracy fault) than would the expanded Clifton Court Forebay.

10 Similarly, the south tunnels and new canal section would be constructed through an area more
11 closely associated with the West Tracy fault compared with the area the Clifton Court Forebay
12 expansion would have covered. Therefore, the potential for failure and consequent potential loss of
13 property, personal injury, or death caused by seismic shaking at the Byron Tract Forebay could be
14 greater with the proposed project compared to that of the approved project.

15 As stated in the analysis of Alternative 4A in Section 9.3.4.2 of the Final EIR/EIS, the results of the
16 seismic study (California Department of Water Resources 2007) show that ground shaking hazards
17 in the Delta are not sensitive to the elapsed time since the last major earthquake (i.e., the projected
18 shaking hazard results for 2005, 2050, 2100, and 2200 are similar).

19 **NEPA Effects:** Seismically induced ground shaking could cause loss of property or personal injury at
20 the proposed project construction sites (including the Byron Tract Forebay pumping plant, the
21 Byron Tract Forebay, and new canal) as a result of collapse of facilities. Facilities lying directly on or
22 near active blind faults may have an increased likelihood of loss of property or personal injury in the
23 event of seismically induced ground shaking.

24 During construction, all active construction sites would be designed and managed to meet the safety
25 and collapse-prevention requirements of the relevant state codes and standards listed under the
26 Alternative 4A analysis in Section 9.3.4.2 of the Final EIR/EIS, and discussed in Appendix 3B,
27 *Environmental Commitments, AMMs, and CMs*, for the anticipated seismic loads.

28 Conformance with these health and safety requirements and the application of accepted, proven
29 construction engineering practices would reduce any potential risk such that construction of the
30 proposed project would not create an increased adverse effect or increased likelihood of loss of
31 property, personal injury or death of individuals. Therefore, there would be no adverse effect.

32 **CEQA Conclusion:** Seismically induced ground shaking that is estimated to occur and the resultant
33 ground motion anticipated at proposed project construction sites, including the Byron Tract
34 Forebay pumping plant, the Byron Tract Forebay, and new canal, could cause collapse or other
35 failure of project facilities while under construction. As described for the approved project, DWR
36 would conform to California Occupational Safety and Health Administration (Cal-OSHA) and other
37 state code requirements, such as shoring, bracing, lighting, excavation depth restrictions, required
38 slope angles, to protect worker safety. Conformance with these standards and codes is an
39 environmental commitment of the project (see Appendix 3B, *Environmental Commitments, AMMs,*
40 *and CMs*).

1 **Incremental Impact:** Under the proposed project, the construction of the Byron Tract Forebay,
2 south tunnels, and canal would involve a slightly greater risk of property, personal injury, or
3 death from seismic shaking during construction than would construction of the approved
4 project, because the Byron Tract Forebay, south tunnels, and canal would be located closer to a
5 potential shaking source (i.e., the West Tracy fault) than would the expanded Clifton Court
6 Forebay. However, conformance with the health and safety requirements described above and
7 the application of accepted, proven construction engineering practices would reduce this risk
8 and there would not be an increased likelihood of loss of property, personal injury, or death due
9 to construction of the proposed project. Therefore, the impact would remain less than
10 significant. No additional mitigation is required.

11 **Impact GEO-2: Loss of Property, Personal Injury, or Death from Settlement or Collapse**
12 **Caused by Dewatering during Construction of Water Conveyance Features**

13 This potential effect could be substantial because settlement or collapse during dewatering could
14 cause injury of workers at the construction sites as a result of collapse of excavations. The risk to
15 and potential effects on life and property as a result of settlement or collapse caused by dewatering
16 during construction would be similar in mechanism and magnitude to those described for the
17 approved project. As with the approved project, settlement of excavations could occur as a result of
18 dewatering at the proposed project construction sites with shallow groundwater. Dewatering can
19 stimulate settlement in excavation and tunneling sites.

20 ***RTM Storage***

21 Proposed changes to RTM locations and other footprint changes would not require any additional
22 dewatering activities.

23 ***Byron Tract Forebay and Conveyance***

24 Dewatering would be required for constructing the new Byron Tract Forebay and conveyance. The
25 settlement could cause the slopes of excavations to fail. Other locations where dewatering would
26 occur during construction of proposed project water conveyance features would be identical to that
27 under the approved project and the potential impacts from construction of the other water
28 conveyance features are identical under both the proposed and approved projects.

29 ***NEPA Effects:*** The hazard of settlement and subsequent collapse of excavations would be evaluated
30 by assessing site-specific geotechnical and hydrological conditions at intake locations, as well as
31 where intake and forebay pipelines cross waterways and major irrigation canals. A California-
32 registered civil engineer or California-certified engineering geologist would recommend measures
33 in a geotechnical report to address these hazards which would conform to applicable design and
34 building codes, guidelines, and standards, as described for the approved project in Final EIR/EIS
35 Section 9.3.4.2, *Alternative 4A*.

36 DWR has made an environmental commitment to also conform to appropriate code and standard
37 requirements to minimize potential risks (see Appendix 3B, *Environmental Commitments, AMMs, and*
38 *CMs*). Therefore, there would be no adverse effect.

1 **CEQA Conclusion:** Settlement or failure of excavations during construction could result in loss of
2 property or personal injury. However, DWR would conform to Cal-OSHA and other state code
3 requirements to protect worker safety, as described for the approved project. DWR has also made
4 an environmental commitment to conform to appropriate codes and standards to minimize
5 potential risks (see Appendix 3B, *Environmental Commitments, AMMs, and CMs*). Additionally, DWR
6 has made an environmental commitment that a geotechnical report be completed by a California-
7 certified engineering geologist, that the report's geotechnical design recommendations be included
8 in the design of project facilities, and that the report's design specifications are properly executed
9 during construction to minimize the potential effects from settlement and failure of excavations.
10 Proper execution of these environmental commitments to minimize potential risks would result in
11 no increased likelihood of loss of property, personal injury or death due to construction of the
12 proposed project.

13 **Incremental Impact:** The dewatering required for constructing the new Byron Tract Forebay
14 and conveyance would be similar to that required for constructing the Clifton Court Forebay,
15 such that the hazard of settlement or collapse of excavations during construction of the
16 proposed project would be similar to that of the approved project. The impact of the proposed
17 project would remain less than significant. No mitigation is required.

18 **Impact GEO-3: Loss of Property, Personal Injury, or Death from Ground Settlement during**
19 **Construction of Water Conveyance Features**

20 ***RTM Storage and Footprint Changes***

21 The risk to and potential effects on life and property as a result of ground settlement of the soil
22 underlying the RTM and the RTM itself during construction would be identical in mechanism and
23 magnitude to those described for the approved project. The geologic substrate (Peat and Muck –
24 Holocene) and the near-surface soils underlying the relocated RTM storage area under the proposed
25 project is the same as under the approved project, as those shown in Figure 9-1, *Geology of the Plan*
26 *Area*. Additionally, the RTM would be identical in composition and would be placed in the same
27 manner, and the RTM slope geometry would be the same under the proposed project as under the
28 approved project.

29 ***Byron Tract Forebay and Conveyance***

30 The potential for settlement of the forebay berms, tunnels, and canal embankment under the
31 proposed project could be greater compared than that of the approved project. The proposed
32 project requires construction of berms to create the Byron Tract Forebay, whereas construction of
33 the Clifton Court Forebay would be achieved largely by excavating into native soils. Unless they are
34 properly engineered and constructed, settlement of the berms and canal embankments could cause
35 these facilities to fail, potentially causing a loss of property, personal injury, or death.

36 **NEPA Effects:** Although the potential effect is expected to be minor, during detailed project design, a
37 site-specific subsurface geotechnical evaluations would be conducted for the Byron Tract Forebay
38 levees, the south tunnels, the canal embankments, and RTM footprints to verify or refine the findings
39 of the preliminary geotechnical investigations. These effects would be reduced with implementation
40 of DWR's environmental commitments and avoidance and minimization measures (see Appendix
41 3B, *Environmental Commitments, AMMs, and CMs*). The results of the site-specific evaluation and the
42 engineer's recommendations would be documented in a detailed geotechnical report, which will

1 contain site-specific evaluations of the settlement hazard associated with the site-specific soil
2 characteristics used to construct the levee berms and canal embankments.

3 As described in Final EIR/EIS Section 9.3.1, *Methods for Analysis*, the measures would conform to
4 applicable design guidelines and standards, such as USACE design measures (see Appendix 3B,
5 *Environmental Commitments, AMMs, and CMs*).

6 The worker safety codes and standards specify protective measures that must be taken at
7 construction sites to minimize the risk of injury or death from structural or earth failure.
8 Conformance to these and other applicable design specifications and standards would ensure that
9 construction of the proposed project would not create an increased likelihood of loss of property,
10 personal injury or death of individuals from ground settlement. Therefore, there would be no
11 adverse effect.

12 **CEQA Conclusion:** Ground settlement of the Byron Tract Forebay berms, canal embankments, and
13 RTM and settlement associated with the south tunnels could result in loss of property or personal
14 injury during construction. However, DWR would conform to Cal-OSHA, USACE, and other design
15 requirements to protect worker safety, as described for the approved project. DWR has made
16 conformance to geotechnical design recommendations and monitoring an environmental
17 commitment (see Appendix 3B, *Environmental Commitments, AMMs, and CMs*). Hazards to workers
18 and project structures would be controlled at safe levels and there would be no increased likelihood
19 of loss of property, personal injury or death due to construction of the proposed project.

20 **Incremental Impact:** The potential for settlement of the Byron Tract Forebay berms and canal
21 embankments and in association with the south tunnels during construction of the proposed
22 project could be greater than settlement resulting from the approved project. The proposed
23 project requires construction of berms to create the Byron Tract Forebay, whereas construction
24 of the expanded Clifton Court Forebay would be achieved largely by excavating into native soils.
25 Consequently, there could be an increased likelihood of loss of property, personal injury, or
26 death. However, hazards to workers and project structures would be controlled at safe levels
27 such that there would be no significant increased likelihood of loss of property, personal injury
28 or death due to construction of the proposed project. Therefore, the impact would be similar to
29 the impact of the approved project and would remain less than significant. No mitigation is
30 required.

31 **Impact GEO-4: Loss of Property, Personal Injury, or Death from Slope Failure during** 32 **Construction of Water Conveyance Features**

33 ***RTM Storage***

34 The risk to and potential effects on life and property as a result of slope failure during construction
35 would be similar in mechanism and magnitude to those described for the approved project.
36 Excavation of borrow material could result in failure of cut slopes and application of temporary
37 spoils and RTM at storage sites could cause excessive settlement in the spoils, potentially causing
38 injury of workers at the construction sites. The potential for slope failure under the proposed
39 project would be identical to that under the approved project. Despite changes to certain RTM
40 locations, the discussion for Alternative 4A in Section 9.3.4.2 of the Final EIR/EIS remains applicable
41 here.

1 **Byron Tract Forebay and Conveyance**

2 Construction of the Byron Tract Forebay and conveyance would create no additional or differing
3 impacts than would the approved project.

4 **NEPA Effects:** The potential effect could be substantial because excavation of borrow material and
5 the resultant cutslopes and potential failure of spoils/RTM fill slopes could cause injury of workers
6 at the construction sites. The potential for slope failure under the proposed project would be
7 identical to that under the approved project.

8 During design, the potential for native ground settlement below the spoils would be evaluated by a
9 geotechnical engineer using site-specific geotechnical and hydrological information.

10 In addition to the risk of slope failure at borrow sites and spoils and RTM sites, there are also
11 potential impacts on levee stability resulting from construction of the proposed project water
12 conveyance facilities. All levee reconstruction/building pad construction would conform to
13 applicable state and federal flood management engineering and permitting requirements.

14 DWR would ensure that the geotechnical design recommendations are included in the design of
15 project facilities and construction specifications and are properly executed during construction to
16 minimize the potential effects from failure of excavations. Conformance with relevant codes and
17 standards would reduce the potential risk for increased likelihood of loss of property or personal
18 injury from settlement/failure of cutslopes of borrow sites and failure of soil or RTM fill slopes
19 during construction. The worker safety codes and standards specify protective measures that must
20 be taken at construction sites to minimize the risk of injury or death from structural or earth failure
21 (e.g., utilizing personal protective equipment, practicing crane and scaffold safety measures). The
22 relevant codes and standards represent performance standards that must be met by contractors and
23 these measures are subject to monitoring by state and local agencies. DWR has made this
24 conformance and monitoring process an environmental commitment (see Appendix 3B,
25 *Environmental Commitments, AMMs, and CMs*).

26 Conformance to these and other applicable design specifications and standards would ensure that
27 construction of the proposed project would not create an increased likelihood of loss of property,
28 personal injury or death of individuals from slope failure at borrow sites and spoils and RTM storage
29 sites. The maintenance and reconstruction of levees would improve levee stability over Existing
30 Conditions due to improved side slopes, erosion control measures (geotextile fabrics, rock
31 revetments, or other material), seepage reduction measures, and overall mass. Therefore, there
32 would be no adverse effect.

33 **CEQA Conclusion:** Settlement or failure of cutslopes of borrow sites and failure of soil or RTM fill
34 slopes could result in loss of property or personal injury during construction. However, because
35 DWR would conform to Cal-OSHA and other state code requirements and conform to applicable
36 geotechnical design guidelines and standards, such as USACE design measures, the hazard would be
37 controlled to a safe level and there would be no increased likelihood of loss of property, personal
38 injury, or death due to construction of the proposed project at borrow sites or at spoils and RTM
39 storage sites. The maintenance and reconstruction of levees would improve levee stability over
40 Existing Conditions due to improved side slopes, erosion control measures, seepage reduction
41 measures, and overall mass.

1 **Incremental Impact:** There would be no incremental impact of construction of the proposed
2 project over the approved project. The potential impact of slope failure and subsequent loss of
3 property, personal injury, or death during construction of the Byron Tract Forebay and
4 conveyance and RTM storage areas during construction of the proposed project would be
5 similar to the impact of the approved project. The impact of the proposed project would remain
6 less than significant. No mitigation is required.

7 **Impact GEO-5: Loss of Property, Personal Injury, or Death from Structural Failure Resulting**
8 **from Construction-Related Ground Motions during Construction of Water Conveyance**
9 **Features**

10 ***RTM Storage***

11 Relocating RTM storage areas would create no additional or differing impacts than would the
12 approved project.

13 ***Byron Tract Forebay and Conveyance***

14 The risk to and potential effects on life and property as a result of structural failure from
15 construction-related ground motions during construction would be similar in mechanism and
16 magnitude to those described for the approved project. Pile driving, shallow tunneling, and other
17 heavy equipment operations would cause vibrations that could initiate liquefaction and associated
18 ground movements in places where soil and groundwater conditions are present to allow
19 liquefaction to occur. The consequences of liquefaction could result in damage to nearby structures
20 and levees. Based on the seismic vulnerability of levees in the vicinity of the Clifton Court Forebay
21 (see Figure 9-4, *Levee Seismic Vulnerability Groups*), work areas at the Byron Tract Forebay would
22 have a similar susceptibility to construction-induced liquefaction as the area for the expanded
23 Clifton Court Forebay. Therefore, the potential for liquefaction under the proposed project would be
24 similar to the potential under the approved project.

25 As with constructing the expanded Clifton Court Forebay, driving of sheet piles would be required to
26 construct the Byron Tract Forebay and conveyance; therefore, the proposed project would not
27 create any additional or differing impacts.

28 ***NEPA Effects:*** The potential effect could be substantial because construction-related ground motions
29 could initiate liquefaction, which could cause failure of structures during construction, which could
30 result in injury of workers at the construction sites. Some of the potential levee effects that could
31 occur during the construction in the absence of corrective measures may include rutting, settlement,
32 and slope movement. The potential for liquefaction under the proposed project would be identical to
33 that under the approved project.

34 During design, the facility-specific potential for liquefaction would be investigated by a geotechnical
35 engineer. The investigations are an environmental commitment (see Appendix 3B, *Environmental*
36 *Commitments, AMMs, and CMs*). In areas determined to have a potential for liquefaction, the
37 California-registered civil engineer or California-certified engineering geologist would develop
38 design strategies and construction methods to ensure that pile driving and heavy equipment
39 operations do not cause liquefaction which otherwise could damage facilities under construction
40 and surrounding structures, and could threaten the safety of workers at the site.

1 Field data collected during design also would be evaluated to determine the need for and extent of
2 strengthening levees, embankments, and structures to reduce the effect of vibrations. These
3 construction methods would conform to current seismic design codes and requirements, as
4 described in Appendix 3B, *Environmental Commitments, AMMs, and CMs*.

5 Should the geotechnical evaluations indicate that certain segments of existing levee roads need
6 improvements to carry the expected construction truck traffic loads, DWR is committed to carry out
7 the necessary improvements to the affected levee sections or to find an alternative route that would
8 avoid the potential deficient levee sections (Mitigation Measures TRANS-2a through 2c). As
9 discussed in Final EIR/EIS Chapter 19, *Transportation*, Mitigation Measure TRANS-2c requires that
10 all affected roadways be returned to preconstruction condition or better following construction.
11 Implementation of this measure would ensure that construction activities would not worsen
12 pavement and levee conditions, relative to Existing Conditions. Prior to construction, DWR would
13 make a good faith effort to enter into mitigation agreements with or to obtain encroachment permits
14 from affected agencies to verify what the location, extent, timing, and fair share cost to be paid by
15 the DWR for any necessary pre- and post-construction physical improvements. Levee roads that are
16 identified as potential haul routes and expected to carry significant construction truck traffic would
17 be monitored to ensure that truck traffic is not adversely affecting the levee and to identify the need
18 for corrective action.

19 DWR has made the environmental commitment that the construction methods recommended by the
20 geotechnical engineer are included in the design of project facilities and construction specifications
21 to minimize the potential for construction-induced liquefaction (see Appendix 3B, *Environmental*
22 *Commitments, AMMs, and CMs*). DWR also has committed to ensure that these methods are followed
23 during construction.

24 Conformance to construction method recommendations and other applicable specifications, as well
25 as implementation of Mitigation Measures TRANS-2a through 2c, would ensure that construction of
26 the proposed project would not create an increased likelihood of loss of property, personal injury or
27 death of individuals due to construction- and traffic-related ground motions and resulting potential
28 liquefaction in the work area. These measures, as written in the Final EIR/EIS, remain adequate
29 without change for dealing with the impacts of the proposed project. Therefore, there would be no
30 adverse effect.

31 **CEQA Conclusion:** Construction-related ground motions and traffic effects could initiate
32 liquefaction, which could cause failure of structures during construction. The impact could be
33 significant. However, because DWR would conform to Cal-OSHA and other state code requirements
34 and conform to applicable design guidelines and standards, such as USACE design measures, in
35 addition to implementation of Mitigation Measures TRANS-2a and TRANS-2b, as well as the
36 maintenance and reconstruction of levees through Mitigation Measure TRANS-2c, the hazard would
37 be controlled to a level that would protect worker safety (see Appendix 3B, *Environmental*
38 *Commitments, AMMs, and CMs*). Further, DWR has made an environmental commitment (see
39 Appendix 3B) that the construction methods recommended by the geotechnical engineer are
40 included in the design of project facilities and construction specifications to minimize the potential
41 for construction-induced liquefaction. DWR also has committed to ensure that these methods are
42 followed during construction. Proper execution of these environmental commitments would result
43 in no increased likelihood of loss of property, personal injury or death due to construction of the
44 proposed project.

1 **Incremental Impact:** There would be no incremental impact of construction of the proposed
2 project over the approved project. The impact of construction-related ground motions and
3 subsequent liquefaction and associated ground movements during construction on loss of
4 property, personal injury, or death resulting from the proposed project would be similar to the
5 impact of the approved project. The impact of the proposed project would remain less than
6 significant with mitigation.

7 **Mitigation Measure TRANS-2a: Prohibit Construction Activity on Physically Deficient**
8 **Roadway Segments**

9 Please refer to Mitigation Measure TRANS-2a under Impact TRANS-2 in Chapter 19,
10 *Transportation*, of the Final EIR/EIS.

11 **Mitigation Measure TRANS-2b: Limit Construction Activity on Physically Deficient**
12 **Roadway Segments**

13 Please refer to Mitigation Measure TRANS-2b under Impact TRANS-2 in Chapter 19,
14 *Transportation*, of the Final EIR/EIS.

15 **Mitigation Measure TRANS-2c: Improve Physical Condition of Affected Roadway Segments**
16 **as Stipulated in Mitigation Agreements or Encroachment Permits**

17 Please refer to Mitigation Measure TRANS-2c under Impact TRANS-2 in Chapter 19,
18 *Transportation*, of the Final EIR/EIS.

19 **Impact GEO-6: Loss of Property, Personal Injury, or Death from Structural Failure Resulting**
20 **from Rupture of a Known Earthquake Fault during Operation of Water Conveyance Features**

21 ***RTM Storage***

22 Relocating RTM storage areas would create no additional or differing impacts than would the
23 approved project.

24 ***Byron Tract Forebay and Conveyance***

25 The risk to and potential effects on life and property as a result of structural failure from rupture of
26 an earthquake fault during operation of the conveyance facilities would be overall similar in
27 mechanism and magnitude to those described for the approved project. The expanded Clifton Court
28 Forebay would have been located in the approximate vicinity of the hanging wall of the West Tracy
29 blind thrust. However, the northern section of the tunnels south of Byron Tract Forebay would
30 intersect with the northern limit of the hanging wall and synclinal axis of the fault (California
31 Department of Water Resources 2011).

32 ***NEPA Effects:*** The proposed project would include overall similar physical/structural components
33 as the approved project; therefore, the effects of the proposed project would be approximately the
34 same as the effects of the approved project. The effect would not be adverse because like the
35 approved project, no active faults extend into the proposed project alignment or footprint.
36 Additionally, although the Thornton Arch and West Tracy blind thrusts occur beneath the proposed
37 project footprint and may be capable of causing surface deformations, they do not present a hazard
38 of surface rupture based on available information, including the AP Earthquake Fault Zone Map

1 showing faults capable of surface rupture (see Figure 9-3, *Active Faults and Historical Seismicity of*
2 *the Bay and Delta Region*).

3 Under the approved project, the expanded Clifton Court Forebay would have been located on the
4 projected hanging wall of the West Tracy blind thrust fault and the axis of the fault's syncline. Under
5 the proposed project, the new Byron Tract Forebay and south tunnels would not be directly located
6 above the West Tracy blind thrust fault and the axis of the fault's syncline (California Department of
7 Water Resources 2011). Therefore, the proposed project may be subject to a slightly lesser hazard of
8 potential fault related effects including surface deformation caused by fault displacement. However,
9 the northern section of the Byron Tract Forebay canal (west of the forebay) may possibly be located
10 on the hanging wall and synclinal axis of the West Tracy blind thrust fault (California Department of
11 Water Resources 2011).

12 There is limited information regarding the depths of the Thornton Arch and West Tracy blind
13 thrusts and seismic surveys would be performed on the blind thrusts during the design phase to
14 determine the depths to the top of the faults. More broadly, design-level geotechnical studies would
15 be prepared by a geotechnical engineer licensed in the state of California during project design.
16 Consistent with the environmental commitments specified in Appendix 3B, *Environmental*
17 *Commitments, AMMs, and CMs*, DWR would ensure that the geotechnical engineer's recommended
18 measures to address adverse conditions would conform to applicable design codes, guidelines, and
19 standards, would be included in the project design and construction specifications, and would be
20 properly executed during construction. Such conformance with design codes, guidelines, and
21 standards is considered an environmental commitment by DWR (see Appendix 3B, *Environmental*
22 *Commitments, AMMs, and CMs*).

23 DWR would ensure that the geotechnical design recommendations are included in the design of
24 project facilities and construction specifications to minimize the potential effects from seismic
25 events and the presence of adverse soil conditions. DWR would also ensure that the design
26 specifications are properly executed during construction.

27 The worker safety codes and standards specify protective measures that must be taken at
28 construction sites to minimize the risk of injury or death from structural or earth failure (e.g.,
29 utilizing personal protective equipment).

30 Conformance to these and other applicable design specifications and standards would ensure that
31 operation of the proposed project would not create an increased likelihood of loss of property,
32 personal injury or death of individuals in the event of ground movement in the vicinity of the
33 project. There would be no adverse effect.

34 **CEQA Conclusion:** With the exception of the West Tracy Trust blind thrust, which may be capable of
35 causing surface deformations, there are no active faults capable of surface rupture that extend into
36 the proposed project alignment or footprint of the Byron Tract Forebay, south tunnels, pumping
37 plant, or canal. However, design-level geotechnical studies would be prepared by a geotechnical
38 engineer licensed in the state of California during project design. The studies would further assess
39 site-specific conditions at and near all the project facility locations, including seismic activity, soil
40 liquefaction, and other potential geologic and soil-related hazards. This information would be used
41 to verify assumptions and conclusions included in the EIR/EIS. Consistent with the project's
42 environmental commitments (see Appendix 3B, *Environmental Commitments, AMMs, and CMs*), DWR
43 would ensure that the geotechnical engineer's recommended measures to address adverse
44 conditions would conform to applicable design codes, guidelines, and standards, would be included

1 in the project design and construction specifications, and would be properly executed during
2 construction. Conformance to these and other applicable design specifications and standards would
3 ensure that operation of the proposed project would not create an increased likelihood of loss of
4 property, personal injury, or death of individuals in the event of ground movement in the vicinity of
5 the project. Therefore, such ground movements would not jeopardize the integrity of the surface
6 and subsurface facilities within the proposed project conveyance alignment or the proposed Byron
7 Tract Forebay and associated facilities.

8 **Incremental Impact:** Because of their closer proximity to the West Tracy fault, the proposed
9 project's Byron Tract Forebay, south tunnels, and canal would be more subject to ground
10 deformation than would the expanded Clifton Court Forebay of the approved project. However,
11 because the West Tracy fault does not appear to be subject to surface rupture, the proposed
12 project would have a similar hazard of loss of property, personal injury, or death from rupture of
13 a known earthquake fault as compared with the approved project during operation of the
14 facilities. The impact of the proposed project would be similar to the impact of the approved
15 project and would remain less than significant. No mitigation is required.

16 **Impact GEO-7: Loss of Property, Personal Injury, or Death from Structural Failure Resulting**
17 **from Strong Seismic Shaking during Operation of Water Conveyance Features**

18 ***RTM Storage***

19 Relocating RTM storage areas would create no additional or differing impacts than would the
20 approved project.

21 ***Byron Tract Forebay and Conveyance***

22 Changes related to constructing the new Byron Tract Forebay and south tunnels instead of the
23 Clifton Court Forebay modifications could result in a greater risk to property, personal injury, or
24 death from structural failure resulting from seismic shaking of conveyance features during
25 operation of the water conveyance features because the Byron Tract Forebay and south tunnels
26 would be located closer to a potential shaking source (i.e., the West Tracy fault) than would the
27 Clifton Court Forebay.

28 Similarly, the new canal section would be constructed through an area more closely associated with
29 the West Tracy fault than would the canal to the Clifton Court Forebay. Therefore, the potential for
30 failure and consequent potential loss of property, personal injury, or death caused by seismic
31 shaking at the Byron Tract Forebay could be greater with the proposed project compared with that
32 of the approved project.

33 Additionally, the Byron Tract Forebay would impound water behind embankments rather than
34 within an excavated basin, as would be the case with the expanded Clifton Court Forebay.
35 Seismically induced failure of the embankments, unless properly engineered and constructed, could
36 cause an uncontrolled release of the impounded water, possibly causing loss of property, personal
37 injury, or death. Similarly, most of the new canal section would consist of embankment fills elevated
38 above the surrounding grade. Failure of the embankments, unless properly engineered and
39 constructed, could cause an uncontrolled release of the impounded water, possibly causing loss of
40 property, personal injury, or death.

1 Further, failure of the Byron Tract Forebay berms as a result of seismic shaking and subsequent
2 catastrophic release of impounded water could pose a greater risk of loss of property, personal
3 injury, and death than with the modified Clifton Court Forebay because the Byron Tract Forebay
4 would be located closer to developed areas than would the Clifton Court Forebay and because more
5 water would be impounded behind berms instead of within an excavation.

6 Earthquake events may occur on the local and regional seismic sources during operation of the
7 proposed project water conveyance facilities. The ground shaking could damage pipelines, tunnels,
8 intake facilities, pumping plants, and other facilities, disrupting the water supply through the
9 conveyance system. In an extreme event of strong seismic shaking, uncontrolled release of water
10 from damaged pipelines, tunnels, intake facilities, pumping plant, and other facilities could cause
11 flooding, disruption of water supplies to the south, and inundation of structures. These effects are
12 discussed more fully in Final EIR/EIS Appendix 3E, *Potential Seismic and Climate Change Risks to*
13 *SWP/CVP Water Supplies*.

14 **NEPA Effects:** This potential effect could be substantial because strong ground shaking could
15 damage pipelines, tunnels, intake facilities, pumping plant, and other facilities and result in loss of
16 property or personal injury. The potential effects of the proposed project would be greater than
17 effects of the approved project. The damage could disrupt the water supply through the conveyance
18 system. In an extreme event, an uncontrolled release of water from the conveyance system could
19 cause flooding and inundation of structures, possibly causing loss of property, personal injury, or
20 death. Please refer to Final EIR/EIS Chapter 6, *Surface Water*, and Final EIR/EIS Appendix 3E,
21 *Potential Seismic and Climate Change Risks to SWP/CVP Water Supplies*, for a detailed discussion of
22 potential flood effects.

23 Except for the new Byron Tract Forebay and conveyance, the design (i.e., pre-cast, reinforced
24 concrete tunnel segments with high performance gaskets) of the underground conveyance facility
25 would decrease the likelihood of loss of property or personal injury of individuals from structural
26 shaking of surface and subsurface facilities along the proposed project conveyance alignment in the
27 event of strong seismic shaking.

28 The new Byron Tract Forebay embankments would be constructed by excavating the embankment
29 foundations down to suitable soil material, dewatering the excavation, and installing a slurry cutoff
30 wall. The embankment fill material would consist either of excavated tunnel material or imported
31 material from borrow sites. Dewatering, moisture conditioning, and compaction of the embankment
32 material would be conducted to the onsite soils. It is assumed that the new Byron Tract Forebay
33 would be subject to the jurisdiction of the Department of Water Resources Division of Safety of
34 Dams (DOSD) because it would store water at an elevation more than 6 feet higher than the
35 surrounding land and therefore would be designed and constructed according to DOSD
36 requirements and be subject to periodic inspections by the DOSD.

37 In accordance with the DWR's environmental commitments specified in Appendix 3B,
38 *Environmental Commitments, AMMs, and CMs*, design-level geotechnical studies would be conducted
39 by a licensed civil engineer who practices in geotechnical engineering. The California-registered civil
40 engineer or California-certified engineering geologist's recommended measures to address this
41 hazard would conform to applicable design codes, guidelines, and standards.

1 DWR would ensure that the geotechnical design recommendations are included in the design of
2 project facilities and construction specifications to minimize the potential effects from seismic
3 events and the presence of adverse soil conditions. Generally, the applicable codes require that
4 facilities be built so that they incur minimal damage in the event of a foreseeable seismic event and
5 that they remain functional following such an event and that the facility is able to perform without
6 catastrophic failure in the event of a maximum design earthquake (the greatest earthquake
7 reasonably expected to be generated by a specific source on the basis of seismological and geological
8 evidence). DWR would also ensure that the design specifications are properly executed during
9 construction. See Appendix 3B, *Environmental Commitments, AMMs, and CMs*.

10 The worker safety codes and standards specify protective measures that must be taken at
11 construction sites to minimize the risk of injury or death from structural or earth failure (e.g.,
12 utilizing personal protective equipment).

13 Conformance to these and other applicable design specifications and standards would ensure that
14 operation of the proposed project would not create an increased likelihood of loss of property,
15 personal injury or death of individuals from structural shaking of surface and subsurface facilities
16 along the proposed project conveyance alignment in the event of strong seismic shaking. Therefore,
17 there would be no adverse effect.

18 **CEQA Conclusion:** The potential impacts of the proposed project would be similar to or slightly
19 greater than impacts of the approved project. Seismically induced strong ground shaking could
20 damage pipelines, tunnels, intake facilities, pumping plant, and other facilities. The damage could
21 disrupt the water supply through the conveyance system. In an extreme event, an uncontrolled
22 release of water from the damaged conveyance system could cause flooding and inundation of
23 structures. (Please refer to Final EIR/EIS Chapter 6, *Surface Water*, for a detailed discussion of
24 potential flood impacts.)

25 **Incremental Impact:** Constructing the proposed project's new Byron Tract Forebay and
26 conveyance instead of the approved project's Clifton Court Forebay modifications could result in
27 a slightly greater risk of loss of property, personal injury, or death from structural failure
28 resulting from seismic shaking during operation of the water conveyance features. This increase
29 would be due to the Byron Tract Forebay's closer proximity to a potential shaking source (i.e.,
30 the West Tracy fault). The shaking could also result in a greater potential for subsequent
31 catastrophic release of impounded water, posing a greater risk of loss of property, personal
32 injury, and death than with the modified Clifton Court Forebay because the Byron Tract Forebay
33 would be located closer to developed areas than would the Clifton Court Forebay and because
34 more water would be impounded behind berms instead of within an excavation. However,
35 through the final design process, which would be supported by geotechnical investigations
36 required by DWR's environmental commitments (see Appendix 3B, *Environmental*
37 *Commitments, AMMs, and CMs*), measures to address this hazard would be required to conform
38 to applicable design codes, guidelines, and standards. Conformance with these codes and
39 standards is an environmental commitment by DWR to ensure that ground shaking risks are
40 minimized as the water conveyance features are operated. The hazard would be controlled to a
41 safe level and there would be a slightly greater likelihood of loss of property, personal injury, or
42 death due to operation of the proposed project. The impact would be the same as the impact of
43 the approved project and would remain less than significant. No additional mitigation is
44 required.

1 **Impact GEO-8: Loss of Property, Personal Injury, or Death from Structural Failure Resulting**
2 **from Seismic-Related Ground Failure (Including Liquefaction during Operation of Water**
3 **Conveyance Features)**

4 ***RTM Storage***

5 Relocating RTM storage areas would create no additional or differing impacts than would the
6 approved project.

7 ***Byron Tract Forebay and Conveyance***

8 The risk to and potential effects on life and property as a result of structural failure resulting from
9 seismic-related ground failure during operation of the conveyance facilities would be similar in
10 mechanism and magnitude to those described for the approved project.

11 Based on the geologic bore hole located closest to the proposed south tunnels that was advanced to
12 the south tunnel depth of 100–150 feet (see bore hole DCBF-DH-012 in Figure 4-2 of Volume 1 of the
13 Conceptual Engineering Report [California Department of Water Resources 2018]), the tunnels
14 would be bored mostly in layers of soil materials ranging from poorly graded sand to fat (i.e.,
15 cohesive and compressible) clay, some of which could be subject to liquefaction and subsequent
16 ground failure if saturated. Therefore, the effects of seismic-related ground failure on the proposed
17 project would be similar to the effects under the approved project. Please refer to Final EIR/EIS
18 Appendix 3E, *Potential Seismic and Climate Change Risks to SWP/CVP Water Supplies*, for a detailed
19 discussion of potential flooding effects.

20 ***NEPA Effects:*** The potential effect could be substantial because seismically induced ground shaking
21 could cause liquefaction, and damage pipelines, tunnels, intake facilities, pumping plant, and other
22 facilities. The damage could disrupt the water supply through the conveyance system. In an extreme
23 event, an uncontrolled release of water from the damaged conveyance system could cause flooding
24 and inundation of structures.

25 In the process of preparing final facility designs, site-specific geotechnical and groundwater
26 investigations would be conducted to identify and characterize the vertical (depth) and horizontal
27 (spatial) extents of liquefiable soil. During final design, site-specific potential for liquefaction would
28 be investigated by a geotechnical engineer. In areas determined to have a potential for liquefaction,
29 a California-registered civil engineer or California-certified engineering geologist would develop
30 design measures and construction methods to meet design criteria established by building codes
31 and construction standards to ensure that the design earthquake does not cause damage to or
32 failure of the facility. The results of the site-specific evaluation and California-registered civil
33 engineer or California-certified engineering geologist's recommendations would be documented in a
34 detailed geotechnical report prepared in accordance with state guidelines, in particular *Guidelines*
35 *for Evaluating and Mitigating Seismic Hazards in California* (California Geological Survey 2008).
36 Conformance with these design requirements is an environmental commitment by DWR to ensure
37 that liquefaction risks are minimized as the water conveyance features are operated (see Appendix
38 3B, *Environmental Commitments, AMMs, and CMs*).

39 Additionally, any modification to a federal levee system would require USACE approval under 33
40 USC 408 (a 408 Permit).

41 The worker safety codes and standards specify protective measures that must be taken at
42 construction sites to minimize the risk of injury or death from structural or earth failure (e.g.,

1 utilizing personal protective equipment). Conformance to these and other applicable design
2 specifications and standards would ensure that the hazard of liquefaction and associated ground
3 movements would not create an increased likelihood of loss of property, personal injury or death of
4 individuals from structural failure resulting from seismic-related ground failure along the approved
5 project conveyance alignment during operation of the water conveyance features. Therefore, the
6 effect would not be adverse.

7 **CEQA Conclusion:** Seismically induced ground shaking could cause liquefaction. Liquefaction could
8 damage pipelines, tunnels, intake facilities, pumping plant, and other facilities, and thereby disrupt
9 the water supply through the conveyance system. In an extreme event, flooding and inundation of
10 structures could result from an uncontrolled release of water from the damaged conveyance system.
11 (Please refer to Final EIR/EIS Chapter 6, *Surface Water*, for a detailed discussion of potential flood
12 impacts.)

13 **Incremental Impact:** The proposed project would have a slightly greater incremental risk of
14 property damage, personal injury, or death from structural failure resulting from seismic-
15 related liquefaction and related failures compared with the approved project. This is because
16 the Byron Tract Forebay, south tunnels, and canal would be closer to a shaking source (West
17 Tracy fault) than would the approved project. However, through the final design process,
18 measures to address the liquefaction hazard would be required to conform to applicable design
19 codes, guidelines, and standards. Conformance with these design standards is an environmental
20 commitment by DWR to ensure that liquefaction risks are minimized as the water conveyance
21 features are operated (see Appendix 3B, *Environmental Commitments, AMMs, and CMs*). The
22 hazard would be controlled to a safe level and there would be no increased likelihood of loss of
23 property, personal injury, or death due to operation of the proposed project. The seismic
24 vulnerability of the proposed project's Byron Tract Forebay and the approved project's
25 expanded Clifton Court Forebay would be similar, as shown in Figure 9-4, *Levee Seismic*
26 *Vulnerability Groups*. The impact would remain less than significant. No additional mitigation is
27 required.

28 **Impact GEO-9: Loss of Property, Personal Injury, or Death from Landslides and Other Slope** 29 **Instability during Operation of Water Conveyance Features**

30 ***RTM Storage***

31 Relocating RTM storage areas would create no additional or differing impacts than would the
32 approved project.

33 ***Byron Tract Forebay and Conveyance***

34 The risk to and potential effects on life and property as a result of slope instability during operation
35 of the conveyance facilities would be similar or slightly greater in mechanism and magnitude to
36 those described for the approved project. The new Byron Tract Forebay would involve construction
37 of embankments, whereas the expanded Clifton Court Forebay under the approved project would
38 involve excavations. Construction of the Byron Tract Forebay canal would also involve construction
39 of embankments.

40 As a result of ground shaking and high soil-water content during heavy rainfall, existing and new
41 slopes that are not properly engineered and natural stream banks could fail and cause damage to
42 facilities.

1 **NEPA Effects:** The potential effect could be substantial because levee slopes and stream banks may
2 fail, either from high pore-water pressure caused by high rainfall and weak soil, or from seismic
3 shaking. Structures built on these slopes could be damaged or fail entirely as a result of slope
4 instability. As discussed in Impact SW-2 in Chapter 6, *Surface Water*, of the Final EIR/EIS, operation
5 of the water conveyance features under the proposed project would not result in an increase in
6 potential risk for flood management compared with Existing Conditions. Peak monthly flows under
7 the proposed project in the locations considered were similar to or less than those that would occur
8 under Existing Conditions. Since flows would not be substantially greater, the potential for increased
9 rates of erosion or seepage are low. For additional discussion on the possible exposure of people or
10 structures to impacts from flooding due to levee failure, please refer to Impact SW-6 in Final EIR/EIS
11 Chapter 6, *Surface Water*.

12 During project design, a geotechnical engineer would develop slope stability design criteria (such as
13 minimum slope safety factors and allowable slope deformation and settlement) for the various
14 anticipated loading conditions. The design criteria would be documented in a detailed geotechnical
15 report prepared in accordance with state guidelines, in particular *Guidelines for Evaluating and*
16 *Mitigating Seismic Hazards in California* (California Geological Survey 2008).

17 Site-specific geotechnical and hydrological information would be used, and the design would
18 conform to the current standards and construction practices. The design requirements would be
19 presented in a detailed geotechnical report. Conformance with these design requirements is an
20 environmental commitment by DWR to ensure that slope stability hazards would be avoided as the
21 water conveyance features are operated (see Appendix 3B, *Environmental Commitments, AMMs, and*
22 *CMs*). DWR would ensure that the geotechnical design recommendations are included in the design
23 of cut and fill slopes, embankments, and levees to minimize the potential effects from slope failure.
24 DWR would also ensure that the design specifications are properly executed during construction.

25 The worker safety codes and standards specify protective measures that must be taken at
26 construction sites to minimize the risk of injury or death from structural or earth failure (e.g.,
27 utilizing personal protective equipment). Conformance to the above and other applicable design
28 specifications and standards would ensure that the hazard of slope instability would not create an
29 increased likelihood of loss of property, personal injury of individuals along the proposed project
30 conveyance alignment during operation of the water conveyance features. Therefore, the effect
31 would not be adverse.

32 **CEQA Conclusion:** Unstable levee slopes and natural stream banks may fail, either from high pore-
33 water pressure caused by high rainfall and uncompacted soil, or from seismic shaking. Structures
34 constructed on these slopes could be damaged or fail entirely as a result of slope instability.

35 However, during the final project design process, as required by DWR's environmental
36 commitments (see Appendix 3B, *Environmental Commitments, AMMs, and CMs*), a geotechnical
37 engineer would develop slope stability design criteria (such as minimum slope safety factors and
38 allowable slope deformation and settlement) for the various anticipated loading conditions during
39 facility operations.

40 DWR would also ensure that measures to address this hazard would be required to conform to
41 applicable design codes, guidelines, and standards. Conformance with these codes and standards is
42 an environmental commitment by DWR to ensure cut and fill slopes and embankments would be
43 stable as the water conveyance features are operated and there would be no increased likelihood of
44 loss of property, personal injury or death due to operation of the proposed project.

1 **Incremental Impact:** The proposed project would have a slightly greater risk of loss of
2 property, personal injury, or death from slope instability during operation of the facilities, as
3 compared with the approved project. This is because construction of the proposed project's
4 Byron Tract Forebay and conveyance would involve higher embankments than those of the
5 approved project's Clifton Court Forebay. Because of the environmental commitments and other
6 measures taken to conform to applicable design codes, guidelines, and standards, the impact
7 would be the same as the impact of the approved project and would remain less than significant.
8 No additional mitigation is required.

9 **Impact GEO-11: Ground Failure Caused by Increased Groundwater Surface Elevations from**
10 **Unlined Canal Seepage as a Result of Operating the Water Conveyance Facilities**

11 ***RTM Storage***

12 Relocating RTM storage areas would create no additional or differing impacts than would the
13 approved project.

14 ***Byron Tract Forebay and Conveyance***

15 The risk of and potential effects of ground failure caused by unlined canal seepage during operation
16 of the conveyance facilities would be identical to those described for the approved project. The
17 proposed and approved projects would not involve construction of unlined canals; therefore, there
18 would be no increase in groundwater surface elevations and, consequently, no effect caused by canal
19 seepage. The canal from the new Byron Tract Forebay leading to the SWP and CVP would either be
20 concrete-lined or earth-lined (California Department of Water Resources 2018).

21 ***NEPA Effects:*** The proposed project would not involve construction of unlined canals; therefore,
22 there would be no increase in groundwater surface elevations and consequently no effect caused by
23 canal seepage. There would be no effect.

24 ***CEQA Conclusion:*** The proposed project would not involve construction of unlined canals; therefore,
25 there would be no increase in groundwater surface elevations and, consequently, no impact caused
26 by canal seepage.

27 **Incremental Impact:** There would be no incremental impact of the proposed project over the
28 approved project. Because neither the proposed project nor the approved project would involve
29 the construction of an unlined canal, the risk of ground failure caused by increased groundwater
30 surface elevations from unlined canal seepage would be identical. There would be no impact and
31 no mitigation is required.

32 **9.3.2 Cumulative Analysis**

33 The Final EIR/EIS found that there was not a potential for the approved project to have a cumulative
34 effect on geologic and seismic hazards and potential adverse effects and significant impacts that
35 could occur to structures and persons in association with construction and operation of the
36 approved project would be restricted to the locations of the construction and the operational
37 activities of these alternatives. These effects and impacts include the potential for loss, injury or
38 death as a result of strong seismic shaking, settlement or collapse caused by dewatering, ground
39 settlement, slope failure (including decreased levee stability from construction and operation
40 activities), seismic-related ground failure (including liquefaction), ground shaking, fault rupture,

1 seiche or tsunami. All of the effects and impacts are mitigated by incorporating standard
2 construction and structural measures into project design and construction.

3 The analysis for cumulative effects for geology and seismicity remains the same as described in the
4 Final EIR/EIS with consideration of the proposed project modifications. Because the risks of loss of
5 property, personal injury, or death associated with the proposed project would not combine with
6 the geologic and seismic risks from other projects or programs in the Plan Area, there would be no
7 cumulative adverse effect or significant impact.

8 **9.4 References Cited**

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