

APPLICATION OF A 2D HYDRODYNAMIC MODEL TO DESIGN OF REACH-SCALE SPAWNING GRAVEL REPLENISHMENT ON THE MOKELUMNE RIVER, CALIFORNIA

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ABSTRACT

In-stream chinook salmon (*Oncorhynchus tshawytscha*) spawning habitat in California's Central Valley has been degraded by minimal gravel recruitment due to river impoundment and historic gravel extraction. In a recent project marking a new direction for spawning habitat rehabilitation, 2450 m³ of gravel and several boulders were used to craft bars and chutes. To improve the design of future projects, a test was carried out in which a commercial modelling package was used to design and evaluate alternative gravel configurations in relation to the actual pre- and post-project configurations. Tested scenarios included alternate bars, central braid, a combination of alternate bars and a braid, and a flat riffle with uniformly spaced boulders. All runs were compared for their spawning habitat value and for susceptibility to erosion. The flat riffle scenario produced the most total, high, and medium quality habitat, but would yield little habitat under flows deviating from the design discharge. Bar and braid scenarios were highly gravel efficient, with nearly 1 m² of habitat per 1 m³ of gravel added, and yielded large contiguous high quality habitat patches that were superior to the actual design. At near bankfull flow, negligible sediment entrainment was predicted for any scenario. Copyright © 2004 John Wiley & Sons, Ltd.

KEY WORDS: river restoration; 2D modelling; salmon; gravel; numerical modelling; salmon spawning; physical habitat

INTRODUCTION

In California salmonid spawning habitat has been degraded or depleted by a plethora of instream human activities and upland land uses (Nehlsen *et al.*, 1991; Moyle and Randall, 1998; Yoshiyama *et al.*, 1998). Dam construction and operation (Kondolf, 1997; Brandt, 2000), gravel extraction (Gilvear *et al.*, 1995; Kondolf *et al.*, 1996), historic gold mining (Harvey and Lisle, 1998), channelization (Nagasaka and Nakamura, 1999), water diversion (Petts, 1996; Douglas and Taylor, 1998), deforestation (Platts and Megahan, 1975; Marks and Rutt, 1997), and intensive agriculture (Soulsby *et al.*, 2000) are specific activities that disrupt healthy stream ecology (Allan and Flecker, 1993; Poff *et al.*, 1997). While California's commercial landings of chinook salmon (*Oncorhynchus tshawytscha*) show wide annual variability, a consistent downward trend in decadal harvest during 1950–2000 from 33 621 to 18 980 tonnes indicates a serious threat to species survival (National Marine Fisheries Service, 2001).

For Sierra Nevada streams, dams prevent salmon from reaching historic spawning sites (Moyle and Randall, 1998). Spawning areas below the lowest impassable dams are now critical to survival of seasonal runs. As a result, maintenance flows are provided during different spawning seasons (Castleberry *et al.*, 1996; Moyle *et al.*, 1998). However, even with a minimal flow regime in place, spawning areas below dams suffer gravel losses in winter floods, have little gravel recruitment, and experience channel changes. Thus, dams have altered flow regimes and created out-of-balance sediment budgets that hurt chinook salmon populations.

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Instream mining has exacerbated the problem by severely depleting available gravel. Mining alters channel geometry and elevation, and involves extensive clearing, flow diversion, sediment stockpiling, and deep pit excavation (Sandecki, 1989). Tens to hundreds of millions of tons of gravel are being removed from California's rivers creating a serious gravel deficit.

Gravel replenishment projects had been undertaken to mitigate severely degraded spawning areas below Sierra Nevada dams in more than 13 rivers in California as of 1992 (Kondolf and Matthews, 1993). The largest of these efforts is on the Upper Sacramento River where gravel replenishment has been conducted from 1979 to 2002. Gravel replenishment has been used to raise beds to pre-regulation elevations, prevent channel incision, and in rare cases to create features such as islands and pool-riffle sequences.

Even though these projects may provide some short-term habitat, the amount of gravel added is a small fraction of the bedload deficit. Gravels placed in the main channel have washed downstream during high flows, requiring continued addition of more imported gravel (California Department of Water Resources, 1995). Also, few objective criteria exist for designing and placing in-channel features. Project failure often results from a lack of understanding of geomorphic processes. In many cases designs have been based on 'folklore' or aesthetic criteria rather than application of hydrology and hydrodynamics (National Research Council, 1992).

Post-construction monitoring of gravel replenishment in California has been limited. One exception was a study of the Merced, Tuolumne, and Stanislaus rivers (Kondolf *et al.*, 1996). Ten sites were excavated and back-filled with smaller gravel to create spawning habitat for chinook salmon from 1990 to 1994, but placed gravel sizes were mobile at high flows recurring at 1.5–4 year intervals. Channel surveys showed that augmented gravels washed out.

Based on limited assessment and uncertain success of gravel projects to date, it is apparent that current strategies are not yielding optimal usable habitat while minimizing gravel losses by flow-induced sediment entrainment. The goal of this study was to test the applicability of a commercial modelling package with a two-dimensional (2D) hydrodynamic model for use in designing fine-scale gravel placement to rehabilitate and augment salmon spawning habitat as well as to reintroduce fluvial complexity. 2D hydrodynamic models quantify depth, velocity, and shear stress at ecologically relevant scales, such as a pool-riffle reach or in the vicinity of a single boulder. When model output is coupled with quantitative estimates of preferred physical habitat conditions, the result is a powerful tool for characterizing instream habitat (Leclerc *et al.*, 1995). If this tool could be used prescriptively, then many potential alternative scenarios could be assessed for habitat quality and geomorphic sustainability prior to project construction.

The test site for this study was a gravel project by the East Bay Municipal Utility District (EBMUD) on the Mokelumne River downstream of Camanche Dam in the Central Valley, California (Figure 1). Commercial software was used to build and compare alternative scenarios with actual pre- and post-project conditions. Habitat suitability indices assessed low-flow chinook salmon spawning habitat, while a sediment mobility index predicted entrainment at near-bankfull discharge. Specific objectives were to: (1) build and analyse 2D models of an alluvial reach before and after gravel replenishment; (2) create four alternative design scenarios—alternate bars, channel braid, alternate bars + braid, and flat with boulders; and (3) compare the pre- and post-project hydrodynamic, habitat, and geomorphic conditions with those of the alternative scenarios. This study provides lessons for future gravel-bar design worldwide.

ECOLOGICAL APPLICATION OF HYDRODYNAMIC MODELS

2D models using depth-averaged Saint-Venant equations are increasingly used for aquatic biology and geomorphology. Their primary advantage over 1D models (e.g. HEC-2 and MIKE-11) for these applications is that they yield fine-scale distributions of velocity vectors (including lateral components) as opposed to ecologically and geomorphically insignificant cross-sectional average downstream speeds. Nodal velocity vectors can be used to estimate local habitat and shear stress conditions. It is very well known that the trade-off for this improvement is the expense of pre-project fine-scale field mapping because the quality of model results is strongly dependent upon the accuracy and spatial resolution of the underlying topographic and parameter measurements as well as the resultant digital elevation model (Leclerc *et al.*, 1995; Ghanem *et al.*, 1996). Most 2D models assume a hydrostatic pressure distribution and are thus incapable of handling substantial vertical accelerations or bed gradients >0.10 (Miller and Cluer, 1998). Lane *et al.* (1999) evaluated the extent 3D models improved predictive ability over 2D

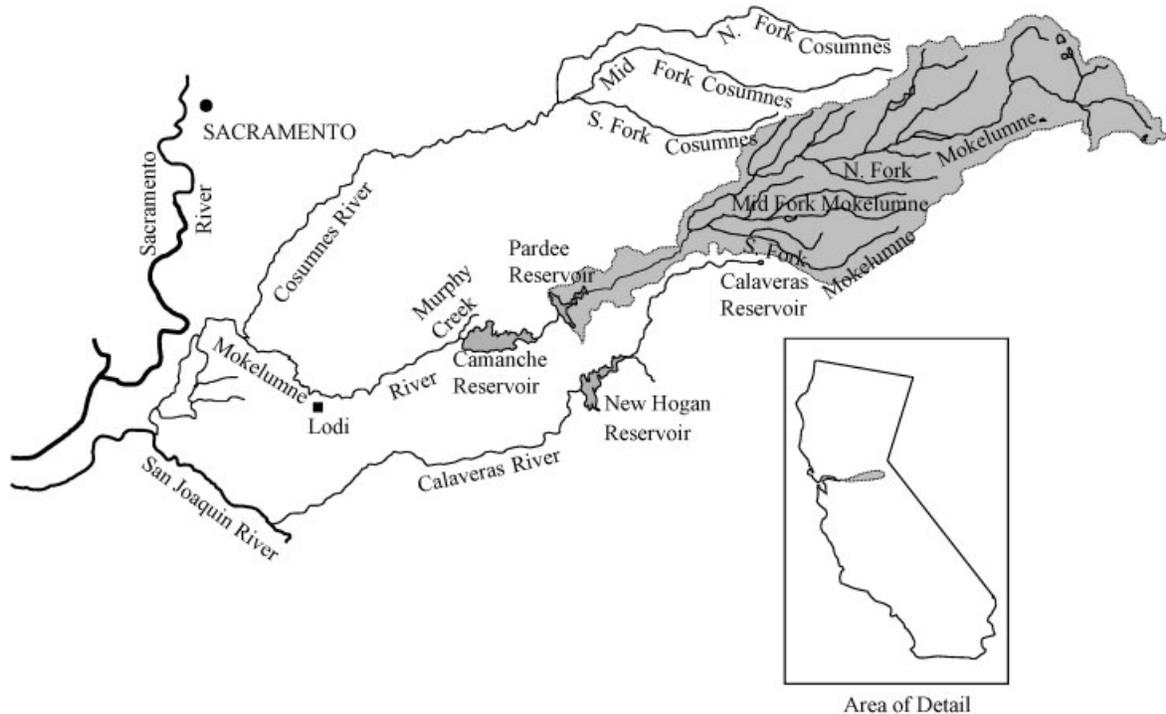


Figure 1. Map of the Mokelumne River basin showing locations of Camanche Reservoir and Murphy Creek

ones. They reported an extreme non-linear sensitivity in 3D models to minor variations in bed geometry and channel network, thus yielding dramatically different flow predictions and bed shear stresses. Given the limits of technology to get field data to calibrate and validate 3D models at the necessary sub-reach scale, 2D models are now the most promising tools for gravel replenishment.

Crowder and Diplas (2000) showed that 2D models can simulate sub-reach-scale stream features with ecological relevance. Small obstructions create velocity gradients, velocity shelters, transverse flows, etc. that were not evident when the obstructions were omitted from their model. Geomorphic applications of 2D models include Miller (1994, 1995), and Cluer (1997).

2D models of habitat require parameters associated with preferred conditions (Vadas, 2000). The Instream Flow Incremental Method is the most widely used approach for combining biology and life habit data of aquatic species with models to predict how water management impacts habitat (Bovee, 1982). Habitat suitability functions defining stream utility have been developed for different life stages of many species. While this approach simplifies complex spatio-temporal interactions, studies have found a significant correlation between predictions and actual community diversity (Gore and Nestler, 1998; Gallagher and Gard, 1999).

STUDY SITE

Mokelumne River basin

The Mokelumne River is a tributary of the San Joaquin River in central California, draining part of Sierra Nevada's western slope (Figure 1). The river drains *c.* 1700 km² and heads at 3050 m. The basin grades from forested mountains in the east to grassy foothills and agricultural floodplains in the west. Precipitation ranges from *c.* 1200 mm in the headwaters to *c.* 510 mm in the lowlands and falls during October to April. Channel widths in the lower river range from 19 to 43 m with a mean of 30 m. The river is connected to its floodplain in the spawning reaches, but is constrained by levees downstream. Adjacent lands are heavily used for recreation and grazing. A



Figure 2. Aerial photo of Mokelumne River below Camanche Dam showing the location of the project reach, the adjacent fish hatchery, and the inlet of Murphy Creek

small tributary, Murphy Creek, enters the river *c.* 1 km downstream of the dam (Figure 1). Detailed hydrology, geology, and ecology for the basin were presented in Wang and Pasternack (2000).

Flow regime

The Mokelumne River has 16 major water projects with four large reservoirs. The two most downstream reservoirs non-passable to anadromous fish are Pardee (completed in 1929) and Camanche (completed in 1964). Camanche aids flood control and river regulation. A fish hatchery built below Camanche to mitigate fishery losses, forced a realignment and excavation of the river (Figure 2). The subsequent altered flow regime stabilized active sediment and enabled in-channel vegetation survival. Changes are documented in historical sources, notably aerial photos. The active channel is now half its former width and overdeepened. The lower Mokelumne begins at Camanche and drains to the Sacramento–San Joaquin Delta.

Hydrologic analysis of pre-dam (1904–1963) and post-dam (1964–1999) annual peak flows below Camanche Dam (USGS ID 11323500) shows the dam's impact. Prior to it, annual peaks exceeded 200 cm for 21 of 57 years. Since 1964, annual peaks have never exceeded 200 cm (Figure 3). Pre-dam mean monthly flow had a typical snowmelt hydrograph (Figure 4a), with highest flow during May and June, after the peak in precipitation (Figure 4b). The post-dam hydrograph shows a significant reduction in the late spring snowmelt runoff below the dam. A flood frequency analysis using annual extreme pre- and post-dam data shows a dramatic reduction in flow for all recurrence intervals after the dam was built (Figure 5). Estimated using Log Pearson III distributions, Q_2 , Q_5 , Q_{10} , and Q_{100} decreased by 67, 59, 73, and 75%, respectively (Wang and Pasternack, 2000). The statistical bankfull discharge ($Q_{1.5}$) prior to Camanche was 120 cm, which is now released only about every five years. Flow out of Camanche has a step hydrograph, with lows near the minimum (4.25 cm) prescribed in the Joint Settlement Agreement for relicensing (Federal Energy Regulatory Commission (FERC), 1998).

Gravel rehabilitation projects

Significant fishery declines during the 1976–1977 and 1987–1992 droughts led to a focus on improving fish habitat. EBMUD and collaborators have replenished gravels below Camanche since 1990 (Table I). FERC (1993) encouraged it as a non-flow alternative for improving habitat, citing lack of gravel recruitment and clogging

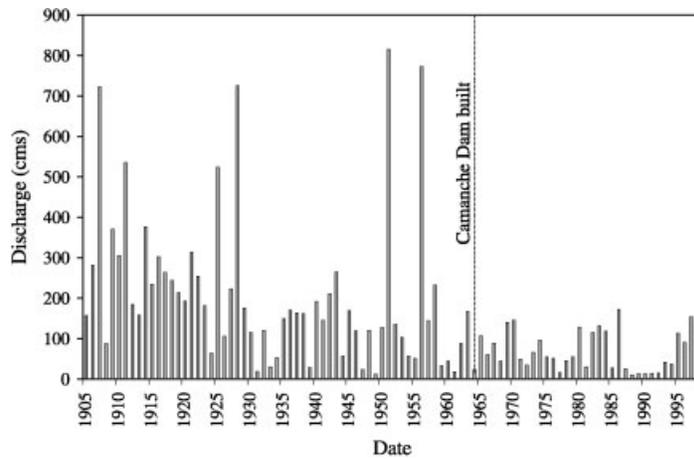


Figure 3. Annual peak flows 1904–1999 from the gauge below Camanche Dam showing significant decrease in peaks with the dam’s management regime

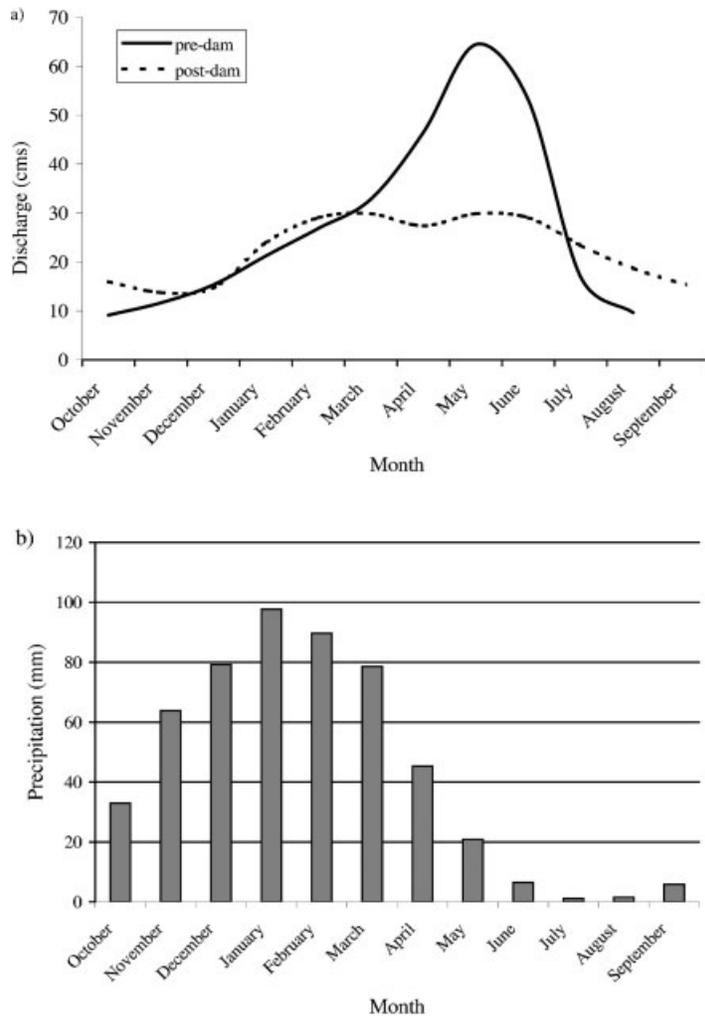


Figure 4. Mean monthly (a) discharge below Camanche Dam for both pre- and post-dam periods and (b) precipitation at Pardee Dam

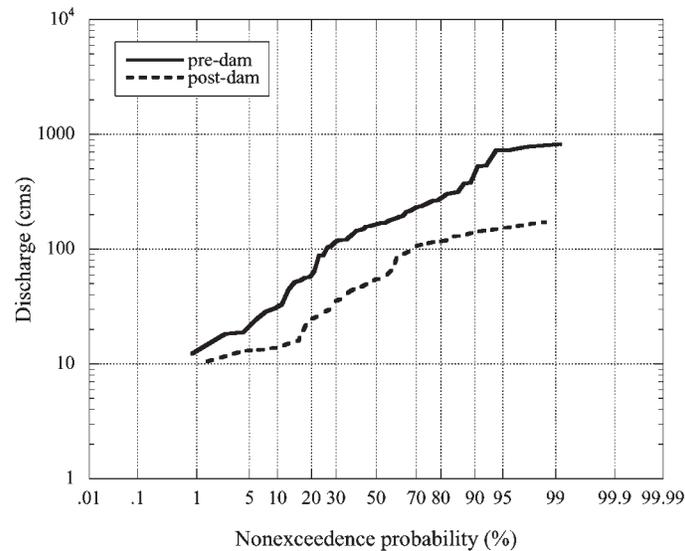


Figure 5. Pre- and post-dam flood frequency curves using annual peak flow series

Table I. History of gravel augmentation/enhancement projects below Camanche Dam

Year	Gravel volume (m ³) ^a	Distance downstream from Camanche Dam (km) ^a
1990	199	0.21
1992	230	0.58
1994	382; 76	0.61; 1.27
1996	268; 268	1.27; 6.88
1997	230; 230	7.84; 9.60
1998	459; 459	0.97; 8.64
1999	2450	0.40
2000	920	1.16

^a Multiple sites in a year separated by semi-colon.

of bed pores. Since 1998, project sites have been monitored for inter-gravel permeability, dissolved oxygen content, water temperature, macro-invertebrate diversity, and redd counts. Some sites will be monitored until 2009.

The site reported here was replenished in fall 1999 (Figure 2). The channel was fairly uniform with a low slope amenable to the model. The right bank was steep with a narrow riparian zone. The low-lying left bank had an accessible floodplain that has heavy recreational use. Initial modelling was done on a 300 m reach past Murphy Creek, but to raise resolution the reach was cut to 150 m ending just upstream of the junction with Murphy Creek.

FIELD METHODS

In fall 1999, EBMUD placed 2450 m³ of washed floodplain gravel in the river below Camanche, creating exposed boulders, chutes, pools, and riffles. Channel topography, water depth and velocity, and substrate sizes were measured for 2D model use. Because this work was initiated after EBMUD's project was underway, there was no chance to measure pre-project velocities. Once the 2D model reasonably replicated post-project flow conditions, four alternative gravel scenarios were designed and compared.

Channel topography

Detailed topography was mapped with 1155 points. Surveying was performed by licensed professionals and was done before and after gravel placement while flow was low. Surveying resolution was high (*c.* 1 point per 3.6 m²). Rather than using a uniform distribution (time-consuming in the field) or many transects (do not capture true topography), care was taken to accurately map bed features with a high density of points measured in the vicinity of natural slope breaks and fewer points over flat surfaces. In addition to coordinates, wet/dry channel boundaries, water surface elevation, and bed exposure were carefully documented.

Observation data

Depth and velocity were measured after the gravel was placed at a typical spawning-season low flow (9.3 cm) and a higher flow (31 cm). The high flow was close to the post-dam $Q_{1.5}$, which is the Q_{bf} suitable for evaluating gravel scour (California Department of Fish and Game (CDFG), 1991). Though the peak for 1999 was 68 cm, this could not be modelled because it spread over the unmapped floodplain.

Post-project depth and velocity were measured at four cross-sections. Endpoints were marked with steel pins to define location and alignment. Endpins were surveyed with a Trimble Pathfinder Pro XRS differential GPS whose resolution was *c.* 0.3 m. GPS coordinates were overlaid in the model to locate the nearest node to each endpin thereby defining comparable cross-sections. Cross-sections 4 (just upstream of placed gravel) and 7 (over gravel bars) were waded at 9.3 cm and sampled every 0.3 m with careful position control. Depths were measured with a stadia rod. At cross-section 4, velocity was averaged over 1 min with a Unidata Starflow depth-averaging ultrasonic Doppler velocity meter ($\pm 1 \text{ mm s}^{-1}$). At cross-section 7, a Marsh-McBirney Flo-mate ($\pm 33 \text{ mm s}^{-1}$) and a depth-setting wading rod were used to estimate average velocity as the point velocity at 0.6 of depth. Cross-sections 1 (upstream boundary for coarse mesh runs) and 10 (below placed gravel) were observed at 31 cm with a cable-mounted USGS Price AA current meter from a flat-bottomed boat locked onto a high-tension steel cable every 1.5 m with careful position control. Standard USGS procedure for calculating depth-averaged velocity from point measurements was used. Positional accuracy and observation resolution were much finer than the scale of bed features (*c.* 5–10 m) and similar to model node spacing.

Substrate size

Wolman pebble counts were conducted before and after replenishment (Kondolf and Li, 1992). Counts were made along three randomly located 30 m longitudinal transects yielding 300 particles. The first particle encountered by the tip of the index finger with closed eyes was sampled. This was done for 100 particles per transect. Each particle was placed into 0.5 phi size class over the range of 3–8 phi including lumped <3 and >8 size fractions (Vyverberg *et al.*, 1996). Cumulative percentages of each class were calculated.

MOKELUMNE MODEL

Model description

The Finite Element Surface Water Modelling System Two-Dimensional Flow in a Horizontal Plane model (FESWMS-2DH v. 2) simulates steady or unsteady 2D surface-water flows, including sub- and super-critical conditions (Froehlich, 1989). FESWMS solves the vertically integrated equations of motion and continuity with a finite element scheme. It uses a robust wetting and drying routine that determines channel boundary location. An element is 'dry' when depth is below a user-defined threshold (0.12 m here) at all of its nodes.

FESWMS was implemented in the Boss International Surface Water Modelling System v. 7.0 (SMS). SMS was used for finite element mesh creation and interpolation of topography to mesh nodes with a TIN-based linear topographic interpolation scheme. Initially a coarse mesh including Murphy Creek was used, but then model area was reduced to 5091 m² with the project and adjacent pool downstream. Elements were *c.* 1 m² with a nodal distance of *c.* 0.5 m (*i.e.* node centred in each element). Pre- and post-project meshes were trimmed close to the known wetted perimeter for each flow to allow for a favourable balance between resolution of relevant bed features and computation time as well as to reduce wetting and drying computations.

Model parameterization

Model parameters were determined using theory and measurement. Camanche outflow was the discharge at the upstream boundary. Water surface elevation was field-surveyed at the downstream boundary (28.1 m for 9.3 cm, 28.6 m for 31 cm). A global constant roughness coefficient (Manning's n) of 0.043 was estimated based on roughness tables for a straight, coarse gravel channel with no vegetation (McCuen, 1989). A spatially explicit algorithm for roughness based on substrate size variations was unwarranted in this study for two reasons. First, the reach had a narrow range of gravel substrate sizes, especially after gravel placement. Second, the form drag of gravel bars caused much more significant roughness. Tests of spatially explicit roughness yielded no significant effect, matching Miller and Cluer (1998).

FEWSMS uses the Boussinesq eddy viscosity to resolve turbulence closure. Eddy viscosity coefficient values that are too high suppress flow separation, whereas values too low cause model instability (Miller, 1994). Eddy viscosity was calculated as $\nu = C u_* R_h$, where $C = 0.6$, $u_* =$ shear velocity, and $R_h =$ hydraulic radius (Froehlich, 1989). Shear velocity was calculated from velocity observations using Einstein's log-velocity equation for turbulent flow over rough beds. Even though calculated eddy viscosities were mostly $< 0.05 \text{ m}^2 \text{ s}^{-1}$, values $< 0.065 \text{ m}^2 \text{ s}^{-1}$ led to model instability, so $0.065 \text{ m}^2 \text{ s}^{-1}$ was used for all pre- and post-project model runs.

Scenario construction

Four alternative scenarios were designed using the SMS Scatter Point Module to compare against the actual ad hoc placement. Even though SMS was not intended for topographic design, it was possible to create topography using its scatter point module. Bed features were first created with artificial points and then pasted over existing topography. Transitions between the pre-existing bed and artificial features were smoothed with additional topographic points.

Given the numerous artificial degrees of freedom in designing alternative reach topography ($c. 30^{25000}$), an objective search for the optimal solution to habitat goals and sediment entrainment constraints was not feasible. Instead, different channel configuration types were tested. The scenarios developed were alternate bars, channel braid, alternate bars + braid, and flat riffle with boulders (Figure 6). All designs except the last used large, coherent bed features that were thought to offer high quality contiguous habitat while also limiting isolated protrusions susceptible to erosion. The downstream half of the model area encompassed a channel-wide pool ($c. 2.52 \text{ m}$ deep). The pool was kept because it was too deep to fill economically and it may provide habitat for other life stages of salmonids. Further, the pool also provided an opportunity to observe any possible important flow dynamics such as recirculating eddies. These eddies are zones of rapid change in flow velocity and direction, which may provide rest sites for spawners between nest-site preparation efforts. Bowl-shaped pool to riffle transitions were also considered beneficial, as they promote stream water downwelling, aid oxygenation of gravels, and are preferentially used by salmonids (Kondolf, 2000).

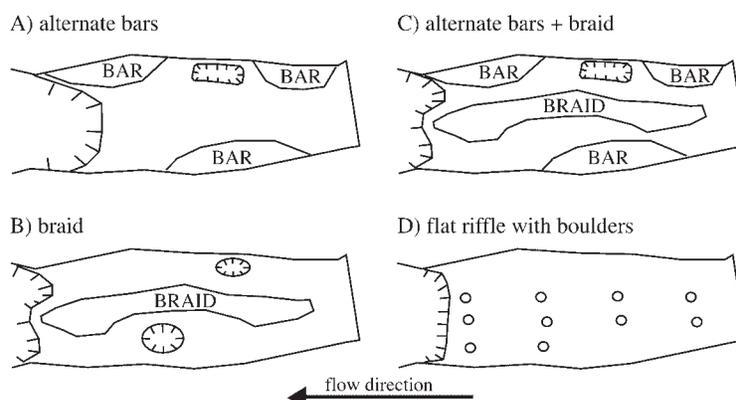


Figure 6. Schematic illustrations of the four alternative gravel placement scenarios. Hatched areas are pools while circles in (D) are boulders. Gravel was used not only to make features, but also to raise bed elevation globally. Detailed topographic maps available in Wang and Pasternack (2000) or upon request

Because the gravel needed for different scenarios varied by up to 30%, comparisons were made based on efficiency—habitat area per cubic metre of gravel added. Gravel volumes were calculated by digital elevation model (DEM) differencing in ArcInfo. Instream features were exposed at low flow by design. Gravel was used not only to make features, but also to decrease water depths globally, as depth was universally greater than the optimal habitat suitability value. Depth and velocity distribution statistics were taken from nodal data sets.

Model parameters were nearly the same as those used in pre- and post-project runs. The roughness coefficient was 0.043 for all cases. Eddy viscosity was $0.065 \text{ m}^2 \text{ s}^{-1}$ for all runs, except for the low flow alternate bars + braid run, for which it was $0.09 \text{ m}^2 \text{ s}^{-1}$ due to model instability. Meshes had equal numbers of nodes and nearly equal numbers of elements.

Scenario comparison

Scenarios were compared using habitat and sediment entrainment criteria. A global habitat suitability index (GHSI) was calculated from velocity (VHSI) and depth (DHSI) suitability curves derived from 98 observed redds at seven sites on the lower Mokelumne (CDFG, 1991). These curves were compared against independent data from the Yuba and Sacramento rivers and found to be similar (CDFG, 1991). The GHSI is specifically for chinook salmon spawning in the lower Mokelumne River under the assumption of perfect substrate quality, which holds when new gravel is placed. DHSI and VHSI range from 0 to 1.13 m and 0 to 1.55 m s^{-1} with optimal conditions at 0.40 m and 0.82 m s^{-1} , respectively. These criteria were combined using $\text{GHSI} = \text{DHSI}^{(0.5)} \times \text{VHSI}^{(0.5)}$. GHSI was classed as poor (0–0.1), low (0.1–0.4), medium (0.4–0.7) and high (0.7–1.0) quality habitat (*sensu* Leclerc *et al.*, 1995). GHSI is calculated at each node and thus cannot assess spatially related habitats. Areas of each GHSI category in each alternative design were calculated and compared.

To assess if placed gravels would wash away at the modelled flows, the ratio of actual to critical velocity was used as a sediment mobility index (SMI). The depth-average critical velocity for movement of the median grain size at a node was calculated using Einstein's log-velocity equation for turbulent flows over rough beds combined with Shield's incipient motion criteria (Garde and Ranga Raju, 1985) assuming a dimensionless shear stress of 0.045:

$$\bar{u}_{\text{critical}} = \left[5.75 \log \left[12.2 \frac{H}{k_s} \right] \right] \left[\frac{0.045(\gamma_s - \gamma_f)d_{50}}{\rho_f} \right]^{0.5} \quad (1)$$

where H = water depth, d_{50} = median bed material grain size, k_s = boundary roughness, γ_s = sediment specific weight, γ_f = fluid specific weight, and ρ_f = fluid density. For the Mokelumne's highly homogeneous bed, d_{50} was taken as k_s (Smart, 1999). This parameter should not be confused with the Z_0 boundary roughness of a more general form of Equation 1, though the two are linearly related (Smart, 1999). Because sediment is unlikely to move under the low flow at which fish spawn (unless flow becomes supercritical), reported SMIs were calculated under near-bankfull flow conditions. $\text{SMI} > 1$ predicts entrainment. For $\text{SMI} < 1$, entrainment potential was divided into low (0–0.33), medium (0.33–0.67), and high (0.67–1.0) categories. Other causes of entrainment that are not quantifiable in this model include fishermen walking on the gravel, gravel consolidation over time, local scour around large woody debris (LWD), and salmon activity.

RESULTS

Gravel placement resulted in significant changes to the channel. Prior to the project, the channel was deep and fairly uniform (Figure 7A) and had a bed composed of poorly sorted, compacted gravels of poor quality for salmon spawning. Grain size frequency analysis showed a pre-project bed d_{50} of $41 \pm 43 \text{ mm}$ (1 standard deviation) (Figure 8). A fraction (13%) of bed particles were $< 8 \text{ mm}$ in diameter, potentially clogging large pores. After the project the channel was shallow and diverse, with several longitudinal bars separated by fast-running chutes (Figure 7B). The far right side of the channel had gravel scattered in small mounds to create exposed bars and riffles under low flow. Exposed boulders were also used to create habitat features. Post-project d_{50} was $48 \pm 21 \text{ mm}$ yielding a homogeneous bed with large interstices (Figure 8). For both pre- and post-project

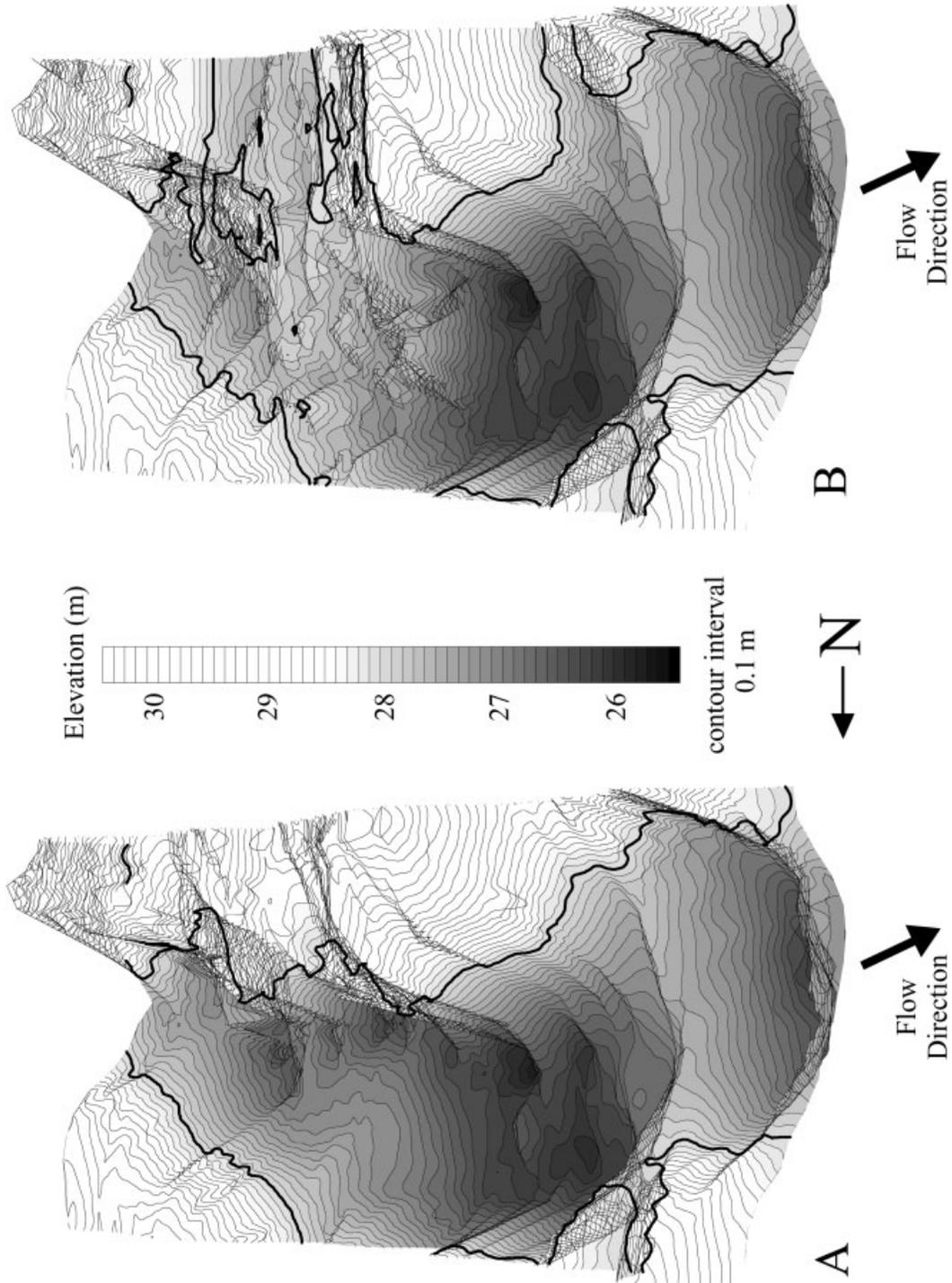


Figure 7. Oblique visualizations looking upstream at (a) pre- and (b) post-project channel geometry. Heavy contour is the 28 m level. Contour interval is 0.1 m

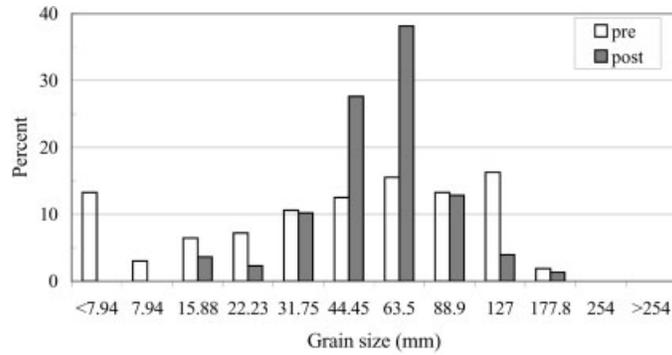


Figure 8. Comparison of bed material grain size distributions before and after gravel replenishment

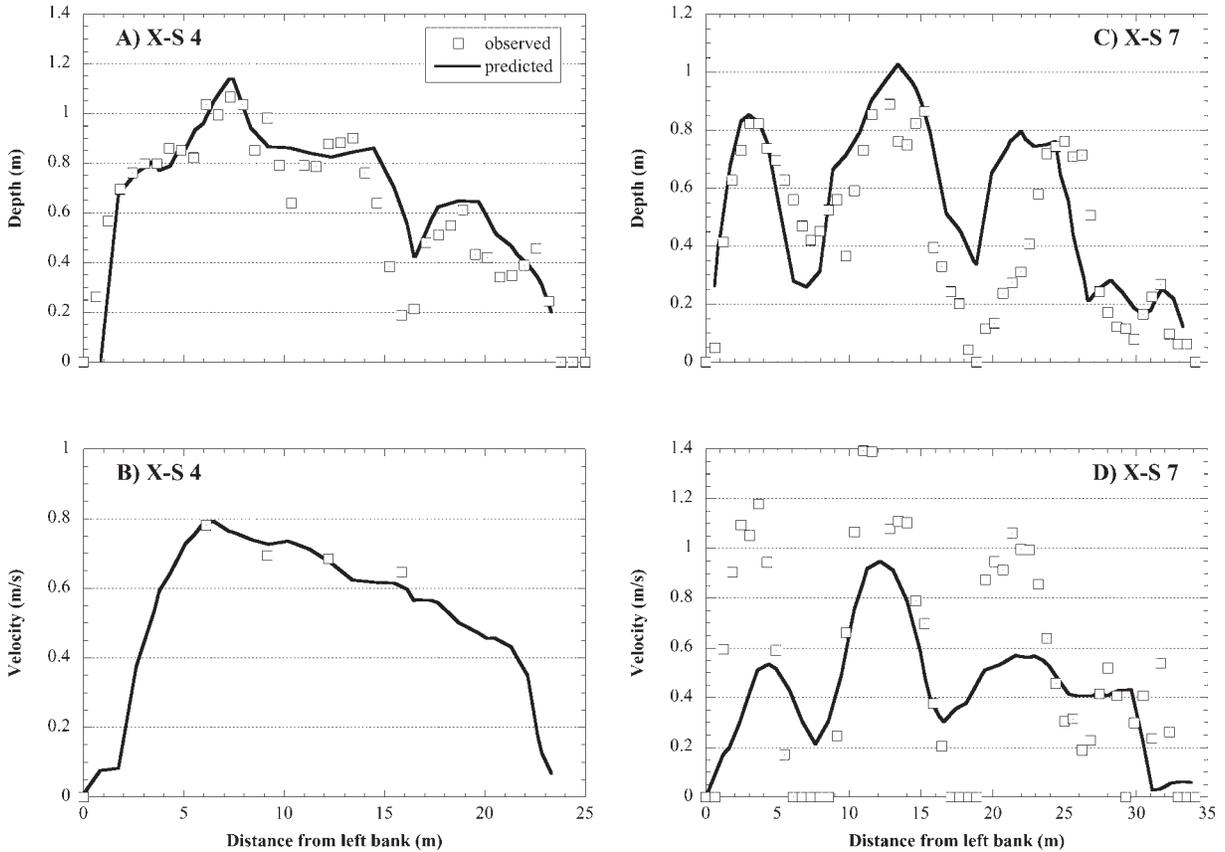


Figure 9. Comparisons of observed versus predicted depths and velocities during a low discharge representative of salmon spawning conditions at cross-sections 4 (A,B) and 7 (C,D)

substrates, the lack of clay, silt, and sand simplified habitat and entrainment conditions so that GHSI and SMI assumptions were reasonable.

Model validation

For the post-project low flow run the model showed very high sensitivity to topographic interpolation error inherent to the simple linear SMS approach. For cross-sections 4 and 7 observed depths ranged from 0 to 1.07 m (Figure 9A,C). The interpolated bathymetry was deeper than observed despite a high density of survey

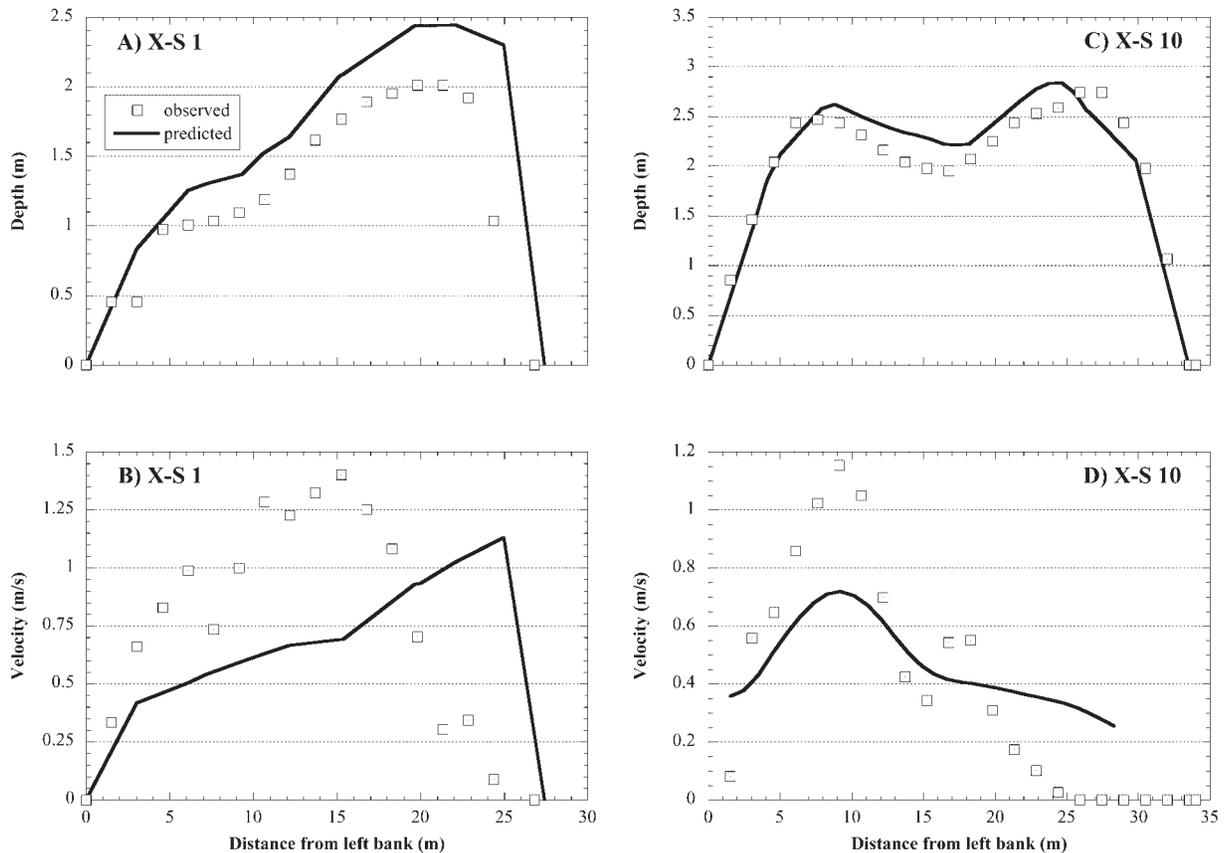


Figure 10. Comparisons of observed versus predicted depths and velocities during a high discharge representative of sediment entrainment conditions at cross-sections 1 (A,B) and 10 (C,D)

points. Cross-section 7 had three bars, three chutes, and a small side channel on the right bank yielding a significant challenge for SMS.

Differences in observed versus predicted velocity primarily stemmed from the topographic interpolation error. Cross-section 4 had an observed peak velocity of 0.78 m s^{-1} . Predicted velocities nearly equalled the five observed values (Figure 9B). Data were lost for the right side of the channel due to battery failure. At cross-section 7, observed velocities were underpredicted in chutes and overpredicted over gravel bars (Figure 9D).

For high flow, model performance at cross-sections 1 and 10 was seriously degraded by LWD and affected by interpolation error. At cross-section 1, validated using a coarse mesh covering that location, depth peaked near the right bank where there was highly branched LWD under water (Figure 10A). Modelled depths were greater than observed, with the worst outcome at the debris. Low flow topographic mapping for model creation avoided the debris whereas high flow observations by boat could not. Observed depths at cross-section 10, where debris was present upstream of the right half of the channel, showed the channel divided into two deep chutes (Figure 10C). Model depths showed some smoothing in relation to observations. Because of the LWD at both cross-sections, velocity transects underpredicted peak velocity and did not capture lateral flow distribution (Figure 10B, D). The effects of LWD will be discussed in detail later.

In summary, validation of the model revealed serious problems with using a simple TIN-based linear interpolation approach on raw topographic data. Where bathymetry was modelled properly and woody debris was absent, the model performed well. These apparent constraints were fully considered in the design phase of the study.

Scenario results

Even though model validation showed that SMS provided poor topographic interpolation in complex terrain so that depth and velocity were underestimated, this deficiency has no impact on comparison of different scenarios with prescribed topography. To compare between real channel configurations and prescribed alternatives required the assumption of accurately interpolated topography. For the pre-project case, the channel was uniform enough that the model reasonably represented topography. For the post-project case, the model was accurate away from bars and chutes but not over such features. Thus, post-project results presented below underestimate details of flow and habitat conditions.

Overall, bed, flow, habitat, and sediment entrainment conditions for the six different bed topographies varied significantly. Model output is illustrated in a series of four maps per scenario: depth, velocity, GHSI at low flow, and SMI at high flow. For the sake of brevity, depth and SMI plots are excluded, though relevant aspects of these plots are summarized below. All plots and raw data sets from this study are available in Wang and Pasternack (2000) or by request.

The pre-project baseline consisted of a uniform channel with low velocities, minimal spawning habitat, and no sediment entrainment (Figure 11A,C). Depths spanned 0–1.82 m, while velocities ranged from 0 to 0.60 m s^{-1} . Velocity vectors were generally parallel and showed little deviation from the longitudinal axis (Figure 11A). Depth controlled GHSI values given low and nearly constant velocities (Figure 11C). The channel was too deep to yield habitat for 74% of the area, leaving a usable fringe along the banks. No high quality habitat was predicted. Monitoring from 1990 to 1998 showed no salmon spawning activity (Setka, 2000), confirming the model prediction. High depths and low velocities uniformly yielded very low SMI values.

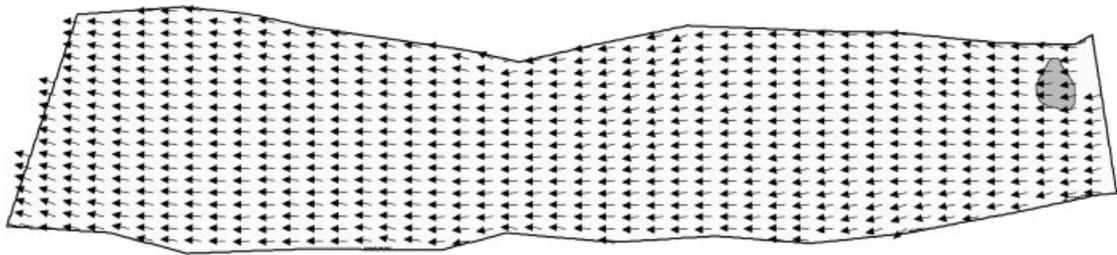
EBMUD's gravel replenishment yielded significantly different and highly complex fluvio-geomorphic conditions (Figure 11B, D). Depth decreased to 0.15–1 m, with the bed exposed over boulders and bar tops. Shallow depths produced faster flows that funnelled between exposed features (Figure 11B). Chute velocities ranged from 1.64 to 2.04 m s^{-1} . A large eddy behind a boulder in both observed and modelled conditions indicated accurate representation of flow pattern, if not flow magnitude. GHSI values were dramatically enhanced due to decreased depth, though the best habitat was highly patchy (Figure 11D). Except for exposed bar tops, the entire project area became usable habitat. Velocities in the fastest chute were too high and produced low quality habitat. Other chutes and some riffle areas produced high quality habitat. Shallower depths and faster velocities increased SMI values, but no entrainment was predicted. Critical velocities were nearly exceeded in cells adjacent to exposed bars. Because the model underpredicted chute velocities, the site was field inspected and these small areas were found to indeed be eroding while other areas were stable as predicted.

In the alternate bars scenario, two small bars were placed on the right bank and one big bar on the left bank (Figure 6A). Opposing bars were connected by riffles; a pool separated the two riffles. Because the reach was short, bars were not intended to mimic natural riffle–pool spacing, but rather to provide simple, coherent bed structures with low risk of erosion during spawning season. The alternate bars decreased channel width, causing flow meandering as well as recirculating eddies downstream of all bars. Bar tops were dry at low flow. Velocities ranged from 0 to 1.28 m s^{-1} (Figure 12A). Despite lacking habitat on exposed bars, the same amount of high quality habitat was predicted as for the actual post-project case (Figure 13A). Habitat was concentrated in three large patches on shallow riffles. SMIs showed no erosion. The highest SMI occurred at the head of the first alternate bar.

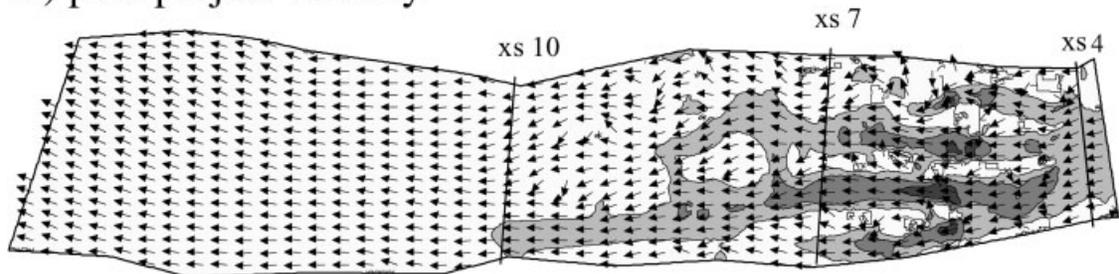
In the braided scenario, an exposed $76 \times 5 \text{ m}$ sinuous braid was placed mid-channel to divide flow into two spawning chutes. A riffle–pool–riffle sequence was made in each chute yielding three pool–riffle transitions promoting water downwelling and substrate oxygenation (Figure 6B). Velocities ranged from 0 to 1.13 m s^{-1} (Figure 12B). Recirculating eddies formed in the shear zone along the braid's upper half. A large two-cell eddy formed over the braid's lower section and a single-cell eddy formed downstream along the right bank. High quality habitat occurred along riffle sections (Figure 13B). Depth and velocity conditions conflicted with each other limiting potential habitat. SMIs were higher over riffles but still too low for entrainment.

The alternate bars + braid scenario had two meandering chutes with riffle–pool–riffle sequences (Figure 6C). The left sequence was highly constricted between the braid and the bar because the left bar was much bigger than the two bars forming the right bank. The size of the bar limited the size and depth of the pool in the left chute. This formed a narrow chute over a riffle that led to a shallow pool and then to another riffle. Once again there were three

A) pre-project velocity



B) post-project velocity



C) pre-project GHSI



D) post-project GHSI

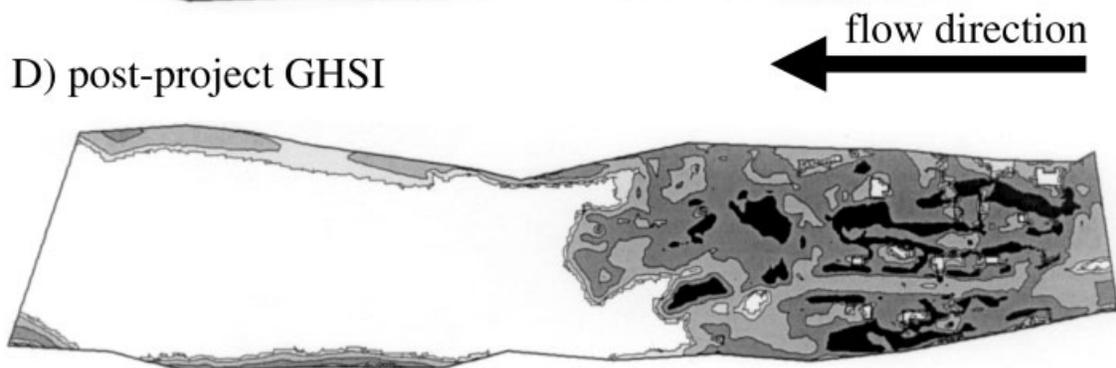
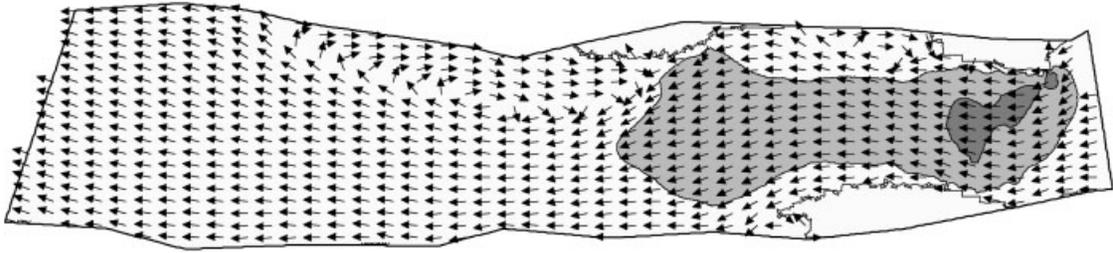
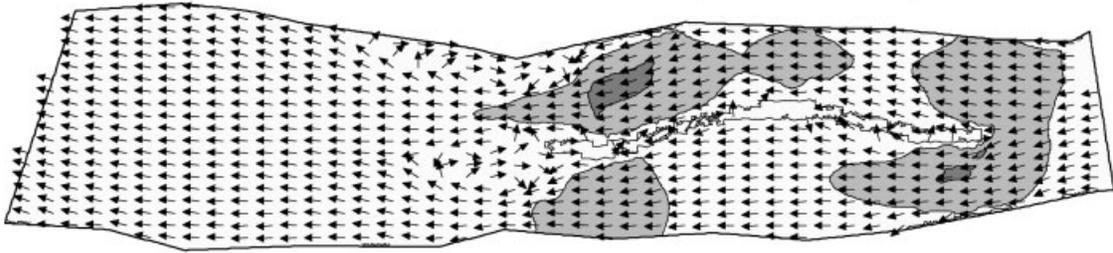


Figure 11. Simulation results for (A) pre- and (B) post-project velocity vectors as well as (C) pre- and (D) post-project GHSI. Validation cross-sections (xs) are shown, except xs 1 upstream of the project reach that was used with a coarser-scale model run. Velocity magnitude is represented with shading such that white = $0-0.5 \text{ m s}^{-1}$, light grey = $0.5-1 \text{ m s}^{-1}$, medium grey = $1-1.5 \text{ m s}^{-1}$, and dark grey = $1.5-2 \text{ m s}^{-1}$. GHSI is represented with shading such that white = 0 (dry areas and deep pools), light grey ≤ 0.1 (very poor quality), medium grey = $0.1-0.4$ (low quality), dark grey = $0.4-0.7$ (medium quality), and black = $0.7-1$ (high quality)

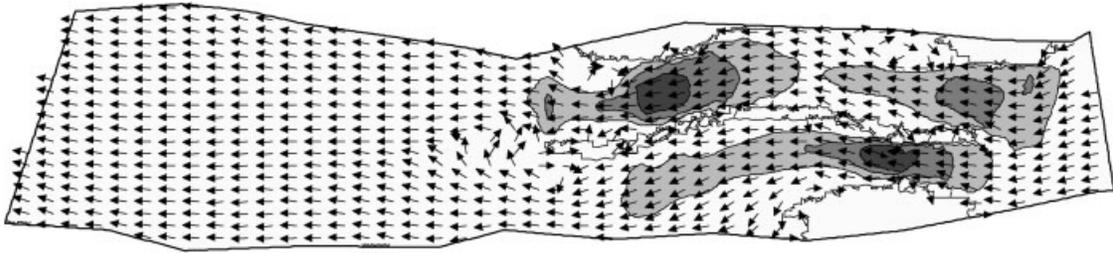
A) alternate bars



B) braid



C) alternate bars + braid



D) flat riffle + boulders

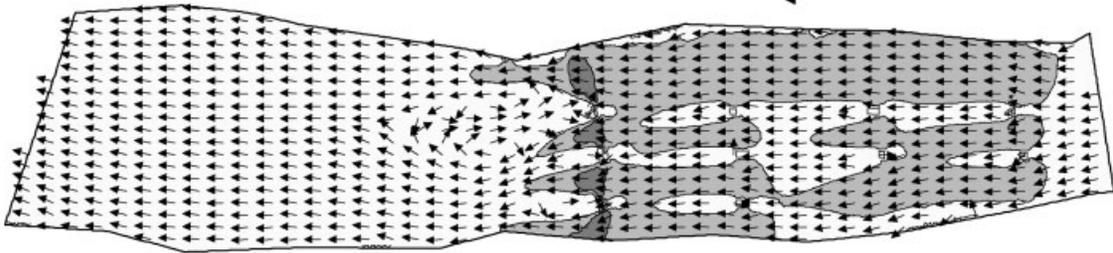
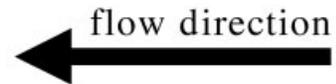
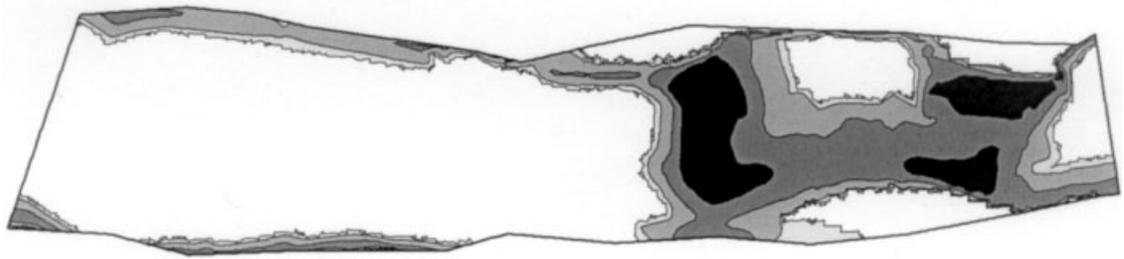
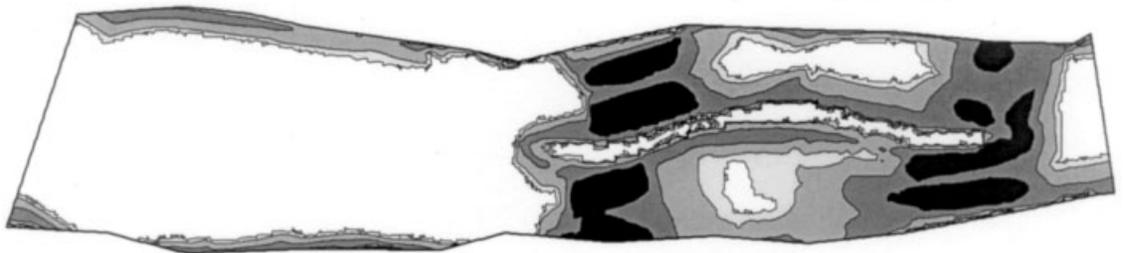


Figure 12. Comparison of velocity magnitude (shading) and direction (arrows) among the four alternative gravel placement scenarios. Dry areas have no arrows. Velocity magnitude is represented with shading such that white = $0-0.5 \text{ m s}^{-1}$, light grey = $0.5-1 \text{ m s}^{-1}$, medium grey = $1-1.5 \text{ m s}^{-1}$, and dark grey = $1.5-2 \text{ m s}^{-1}$

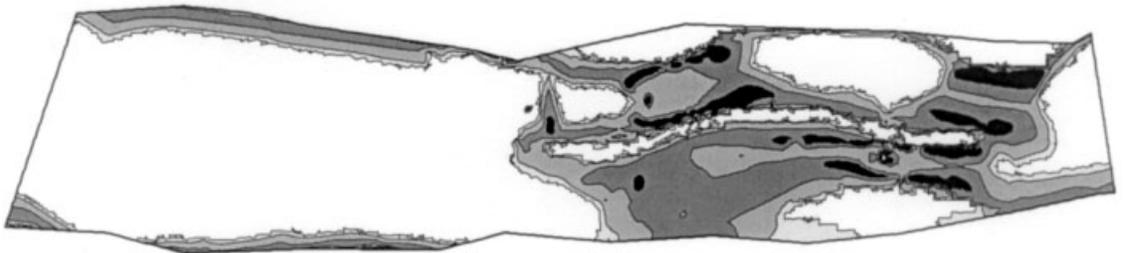
A) alternate bars



B) braid



C) alternate bars + braid



D) flat riffle + boulders

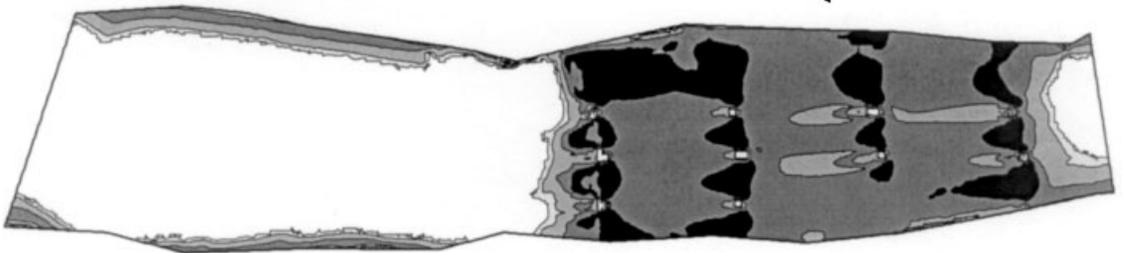


Figure 13. Comparison of GHSI values among the four alternative gravel placement scenarios. White = 0 (dry areas and deep pools), light grey ≤ 0.1 (very poor quality), medium grey = 0.1–0.4 (low quality), dark grey = 0.4–0.7 (medium quality), and black = 0.7–1 (high quality)

pool-exit slopes promoting substrate oxygenation. Velocity was fastest over the riffle sections, with a peak of 2.0 m s^{-1} (Figure 12C). Eddies formed behind each exposed bar. In comparison to the braided scenario, chute asymmetry changed the mid-channel eddy to a single cell. The large number of exposed features built into this design resulted in too much unusable area (Figure 13C). Deep pools also provided no habitat. Few high quality

habitat areas existed, as optimal depth coincided with excessive velocity, while optimal velocity occurred in excessive depth. Relatively high SMI values were concentrated at the head of the first alternate bar.

For the flat riffle + boulders scenario, gravel was set uniformly over the site. Ten boulders were spaced evenly over the left 67% of the channel to constrict flow, create high velocities, and obtain small recirculating eddies providing resting locations for juveniles and adults (Figure 6D). Boulders were *c.* 3 m in diameter and exposed at low flow. The upstream end of the riffle had a broad pool-exit slope for water downwelling. The fastest velocities occurred in the constrictions between boulders, typically $0.5\text{--}1.22\text{ m s}^{-1}$ (Figure 12D). Velocities were lowest behind boulders (*c.* $0.01\text{--}0.5\text{ m s}^{-1}$). Due to the uniformity of gravel and lack of exposed areas, this scenario yielded the most habitat area, though the most gravel was used too (Figure 13D). Boulder symmetry produced regularly distributed high quality habitat patches in the optimal velocity chutes between boulders. Highest SMIs occurred in grid cells just upstream of each boulder.

DISCUSSION

Several studies explain the effect of sparse topographic data on 2D modelling (Anderson and Bates, 1994; French and Clifford, 2000; Marks and Bates, 2000), but few have addressed the significance of DEM-generation limitations within 2D modelling packages. This project had dense data surveyed by professionals yielding a high quality baseline at the scale of habitat-relevant features. This assisted the design of alternatives and enabled a test of SMS and FESWMS in comparing designs on the basis of habitat and sediment entrainment patterns. Unfortunately, SMS v. 7.0 proved unable to generate a digital elevation model that accurately reflected complex natural topography, even with a node spacing of 0.5 m. Lacking that, FESWMS could not match observed conditions. SMS does not have enough tools for accurate terrain modelling. While its topographic interpolation scheme is identical to other DEM software, what is needed is better management of large and layered topographic datasets as well as options to apply accepted civil engineering methods that manually adjust the terrain model to yield an accurate representation as can be done with many CAD programs, such as AutoDesk's AutoCAD. DEMs made using engineering software may be imported into SMS and used with a 2D model to yield more computationally stable and accurate simulations. Thus, in addition to collecting dense topographic datasets, one must work hard at DEM generation to obtain an accurate representation of the bed for 2D modelling.

Even with an accurate terrain model, a serious problem was encountered in representing flow conditions near LWD. While woody material was not widespread, two large pieces altered the flow pattern of the model validation cross-sections. Resulting errors affected habitat quality estimates and location as well as potential for sediment entrainment. As large woody debris can provide significant instream habitat (Abbe and Montgomery, 1996; Merz, 2001) and is part of channel rehabilitation, this poses a limitation on the use of a 2D model where wood is present. Changing local flow parameters to better mimic woody debris can help a model match observed conditions, but this is not possible in a design mode predicting future velocities under alternatives. Further, habitat provided by woody debris is not accounted for in habitat suitability indices, so the use of model results to predict habitat conditions would be tenuous.

Gravel replenishment yielded a dramatic change in flow conditions and thus in quantity and quality of habitat. Mean depth decreased from 1.38 m to 1.09 m, while predicted mean velocity increased from 0.21 to 0.34 m s^{-1} (Table II). More importantly, the range of predicted velocities increased from $0\text{--}0.6$ to $0\text{--}2.07\text{ m s}^{-1}$. Because the model underestimated actual velocity (Figures 9 and 10), and higher velocity would have yielded higher GHSI values, habitat quality was underestimated. Even so, the site went from having no high quality habitat to having at least 362 m^2 of it. Total usable habitat changed from 945 to over 2350 m^2 , with the largest increase occurring for medium quality habitat. The efficiency of the gravel project was $0.96\text{ m}^2\text{ m}^{-3}$. Whereas no spawning occurred in the reach in prior years, after gravel placement redds were observed on-site within two months. The follow year there were 29 redds on-site (out of 987 for the whole lower river). These changes show gravel replenishment significantly improves quantity and quality of habitat, even without an objective design process.

Comparing the predicted quantity of habitat under different scenarios showed that many produced similar outcomes (Table III). Even though designed bed features had large exposed areas that provided no habitat, their structures yielded more efficient habitat generation in between so overall gravel efficiency was $0.91\text{--}0.99\text{ m}^2\text{ m}^{-3}$, which was comparable to the ad hoc project. The only scenario that resulted in a very poor utilization of gravel

Table II. Depth and velocity distributions for each scenario

Scenario	Mean ^a depth (m) ^b	Velocity (m s ⁻¹)		Gravel (m ³)
		Mean ^a	Range	
Pre-1999	1.38 ± 0.52	0.21 ± 0.08	0–0.60	n/a
Post-1999	1.09 ± 0.66	0.34 ± 0.32	0–2.07	2450
Alternate bars	1.15 ± 0.68	0.29 ± 0.27	0–1.31	1911
Braided	1.10 ± 0.64	0.31 ± 0.24	0–1.14	2370
Bars + braided	1.09 ± 0.70	0.30 ± 0.31	0–2.03	2675
Flat riffle + boulders	1.08 ± 0.62	0.37 ± 0.24	0–1.37	2675

^aMean ± 1 SD of spatial data from whole area of scenario mesh.

^bDepths for all scenarios depths ranged from 0 to 2.52 m.

Table III. Estimated area and gravel efficiency of each quality of spawning habitat

Scenario	Low			Medium			High			Total		
	%	m ²	m ² m ⁻³	%	m ²	m ² m ⁻³	%	m ²	m ² m ⁻³	%	m ²	m ² m ⁻³
Pre-1999	11	581	n/a	7	364	n/a	0	0	n/a	17	945	n/a
Post-1999	16	840	0.34	23	1146	0.47	7	362	0.15	46	2348	0.96
Alternate bars	13	671	0.35	14	720	0.38	7	356	0.19	34	1747	0.91
Braided	17	879	0.37	17	843	0.36	9	459	0.19	43	2181	0.92
Bars + braided	16	792	0.30	17	856	0.32	3	177	0.07	36	1825	0.68
Flat riffle + boulders	9	465	0.17	32	1617	0.60	11	560	0.21	52	2642	0.99

was the alternate bars + braid scenario, which generated 0.68 m² m⁻³. In that case, the exposed areas of the alternate bar and braid features were too large. The flat riffle + boulders scenario used the most gravel, but was also the most efficient at generating habitat.

Focusing on the quantity and distribution of high quality habitat areas, the alternatives yielded much more favourable results than the actual design (Table III). According to EBMUD's subsequent spawning survey, 10% of redds were superimposed (Setka, 2000), so habitat connectivity and patch size were used to compare scenarios. The flat riffle scenario yielded the most high quality habitat area (560 m²) and had many sizable patches. Despite the small amount of gravel used in the alternate bars scenario, it had nearly the same quantity of high quality habitat as the actual design, yielding improved gravel efficiency. In this case, high quality habitat was focused into three patches that were the largest sizes of all patches in all scenarios, which is a desirable outcome. Similarly, the braided scenario resulted in five large patches and the secondmost area of high quality habitat. The alternate bars + braided scenario yielded very little high quality habitat despite the large amount of gravel used.

Design scenarios may also be differentiated on the basis of other qualitative considerations. Geist and Dauble (1998) proposed that salmon redd distribution in large alluvial rivers was a function of the interaction of surface water and groundwater via the hyporheic zone. In contrast to the actual design, the alternative designs explicitly include zones of surface water downwelling into the gravel. Both the braided and alternate bars + braided scenarios had the most downwelling zones built into them. Similarly, 2% of observed redds in the follow-up survey were along boulders (Setka, 2000), which provide refugia for salmonids and add fluvial complexity, though they tend to promote gravel entrainment (House and Boehne, 1985; Fuller, 1990). Crowder and Diplas (2000) showed that SMS can model flow conditions around boulders, and that has been corroborated here. Boulders were essential to the flat riffle scenario for controlling flow and generating habitat patches. Boulders can be placed in any scenario and will be explored further in future studies. Even without boulders, alternative scenarios had exposed bars that created eddies and refugia.

Because the flat riffle scenario was designed to have optimal flow conditions for a single design discharge, it yielded the best outcome. However, Camanche outflow during the spawning season can vary from 14 to c. 46 cm

between years. Under such a range, matching a single design flow in any given year is unlikely unless dam operations specify it. Deviation from the design discharge would yield little to no usable habitat. In contrast, the other design approaches would provide a suitable area of habitat under a range of flows because of their topographic complexity.

No sediment entrainment for the median grain size was predicted for any scenarios at near-bankfull flow (31 cm). The scenarios with sites that were approaching an entrainment condition were the actual design and the flat riffle with boulders. In both cases potential entrainment was located at the heads of boulders and bars. Ideally, the model would be used to simulate higher flows, but above-bankfull conditions induce flooding of the adjacent forested floodplain, which could not be modelled at a reasonable cost. In 1986 the California Department of Fish and Game performed a simple sediment study for the Mokelumne River using HEC-6 (CDFG, 1991). At a cross-section just upstream of the site, they predicted that at *c.* 30 cm, grain sizes of ≤ 4.6 mm would be entrained. Even at a flow of *c.* 142 cm, HEC-6 predicted that only sizes smaller than 12.5 mm would be transported. As the added gravel had a median size of 48 mm, constructed features should be well protected against flow-induced gravel losses. Based on observation, a greater concern may be erosion by supercritical flow at extremely low flow depths.

In this study it was found that an off-the-shelf 2D modelling package could be used to design and compare alternative scenarios for gravel rehabilitation of salmon spawning sites on the basis of multiple quantitative and qualitative habitat and sediment mobility criteria. When an accurate DEM is generated using a high-density topographic survey and professional design engineering software, then a hydrodynamic model can accurately simulate spatial patterns in velocity, depth, sediment entrainment, and spawning habitat in streams. By comparing these variables for six different channel configurations it was possible to distinguish design features that generated and enhanced physical habitat from those that were counterproductive. The strongest conclusion reached is that ad hoc gravel replenishment yields highly patchy habitat conditions that are less gravel-efficient and more likely to erode. Even with large areas of exposed gravel, alternative designs matched or exceeded the actual design. The use of exposed gravel at the design flow is warranted when higher than normal discharges are planned for. In this case, flow may be lower than the design discharge, so it would have been better to keep bars submerged. With that additional area, alternatives would probably greatly exceed the actual design in habitat area and gravel efficiency. Among alternatives, the braided scenario would have been selected for construction because of its large amount of total and high quality habitat, its gravel efficiency, its generation of multiple oxygenation zones and one- or two-cell recirculating eddies, and its lack of sediment entrainment. This choice is probably unique to this site, and many other possible alternatives were not considered in the design phase.

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